REte dei Laboratori Universitari di Ingegneria Sismica

Modelli e tecniche nei progetti di intervento

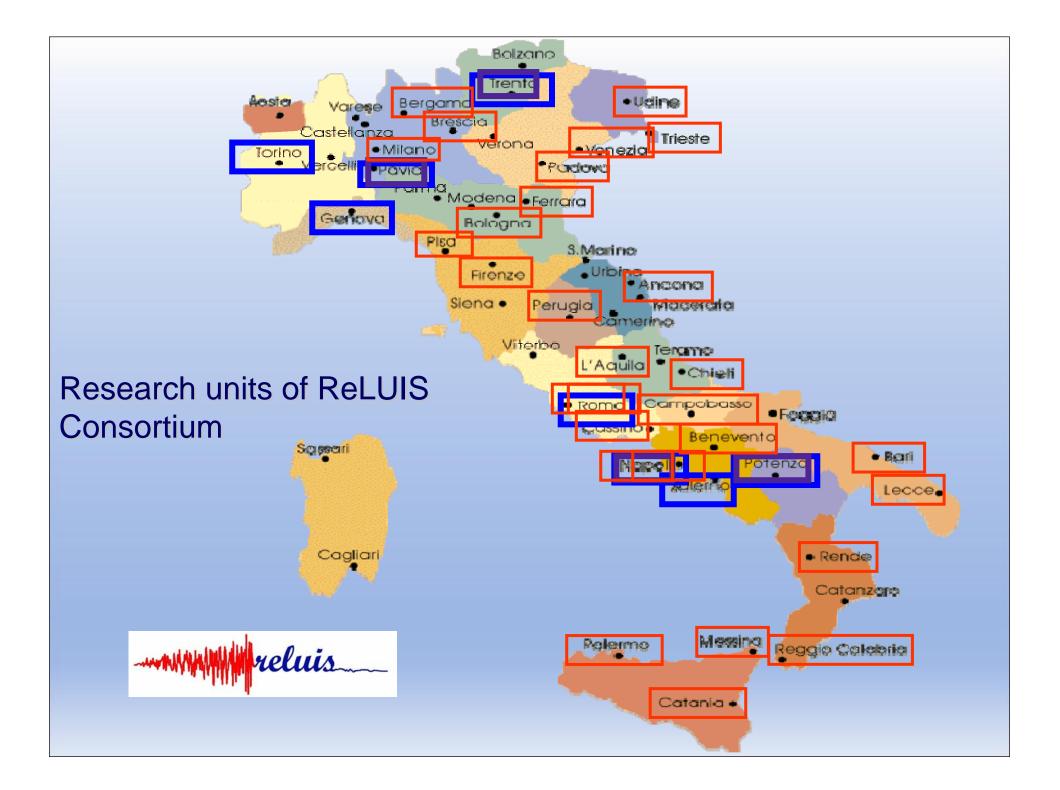
Gaetano Manfredi

Chairman of Consortium ReLUIS

Dipartimento di Ingegneria Strutturale Università degli Studi di Napoli Federico II



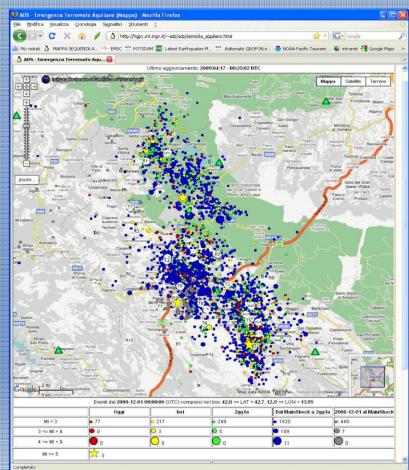
	University of Napoli Federico II AMRA	2 GdL, Dual table system:
		2 X 3x3 mq, 20tx2,5m, 1.0 m/s
	University of Pav	ia 1 GdL, Large mass:
	Eucentre	5x7 mq, 300tm, 1-1.5 m/s
	University of Basilic	ata Large shear wall: Pseudodynamic test
		6 GdL: 4x4 mq, 9.1t, 0.5 m/s
	UTS MAT	Large shear wall
	Università of Trento	Pseudodynamic test



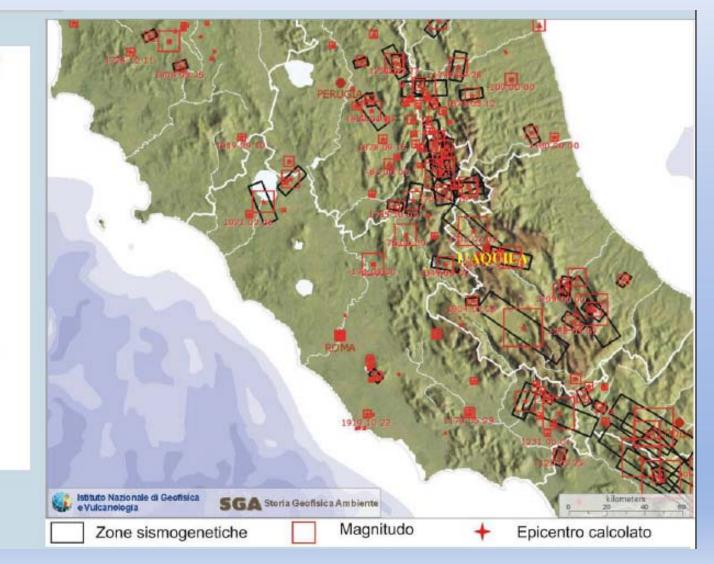
April 6 2009 H. 3.32 a.m.



MI 5.8 – Richter Mw 6.3



Historic seismicity



TERREMOTI ABRUZZESI CON INTENSITA' >IX •Sulmona del 1315 (Sulmona, I = IX)

•Abruzzo 1349 (I = IX-X)

Aquila del 1461
 (L'Aquila, I = X)

Amatrice 1639
 (Amatrice, I = X)

Norcia/Aquila del 1703
 (Norcia–L'Aquila, I = XI)

•Maiella del 1706 (Maiella, I = IX-X)

Avezzano del 1915
 (Avezzano, I = XI)



REte dei Laboratori Universitari di Ingegneria Sismica

Damage to RC Buildings

Columns





Columns

Shear Failure at the rectangular column end.

The stirrups spacing seems to be higher than 200mm.

Poor concrete quality



Columns

Shear Failure



Columns

Shear Failure on the short columns due to the basement wall



Beam-Columns Joints

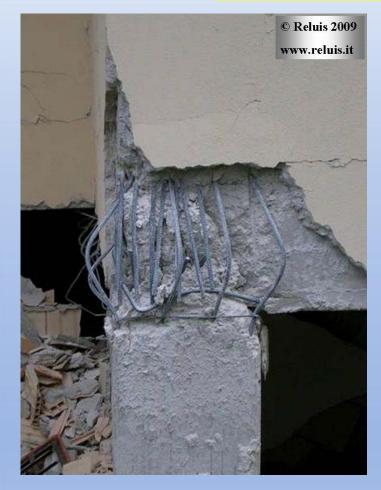




Lack of stirrups in the joint

Buckling of longitudinal rebars from column to the joint

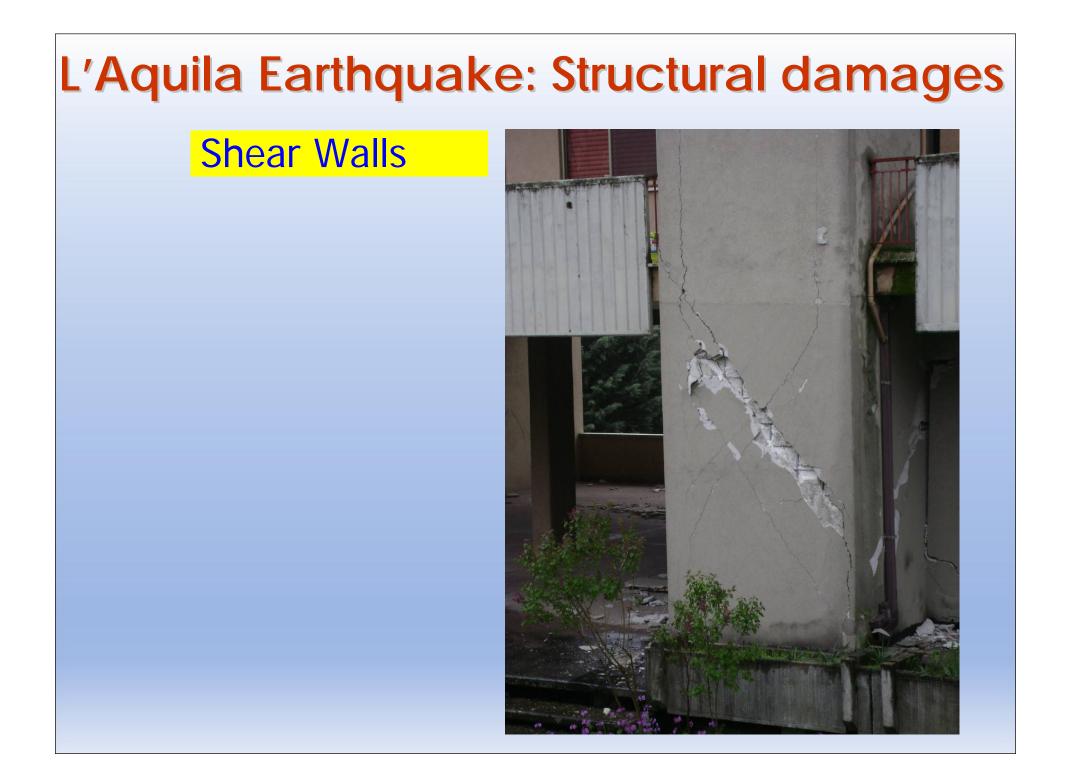
Beam-Columns Joints



No stirrups in the joint

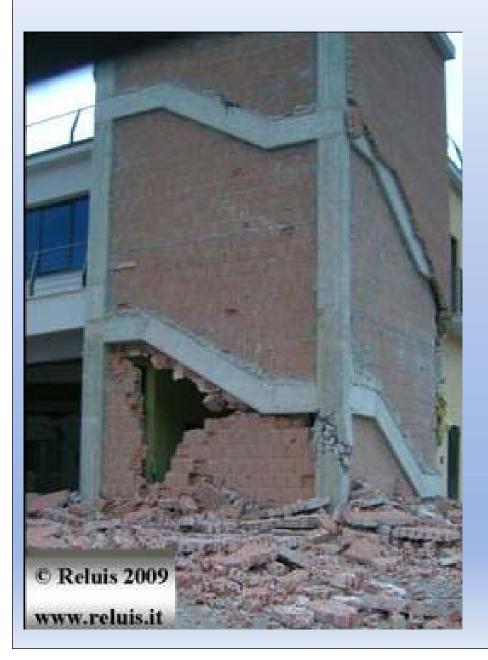


Poor concrete quality



Shear Walls

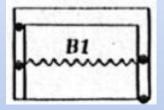




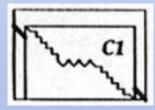
Staircase



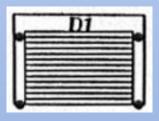
Infills



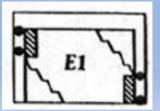
Failure due to horizontal crack at partition's mid-height



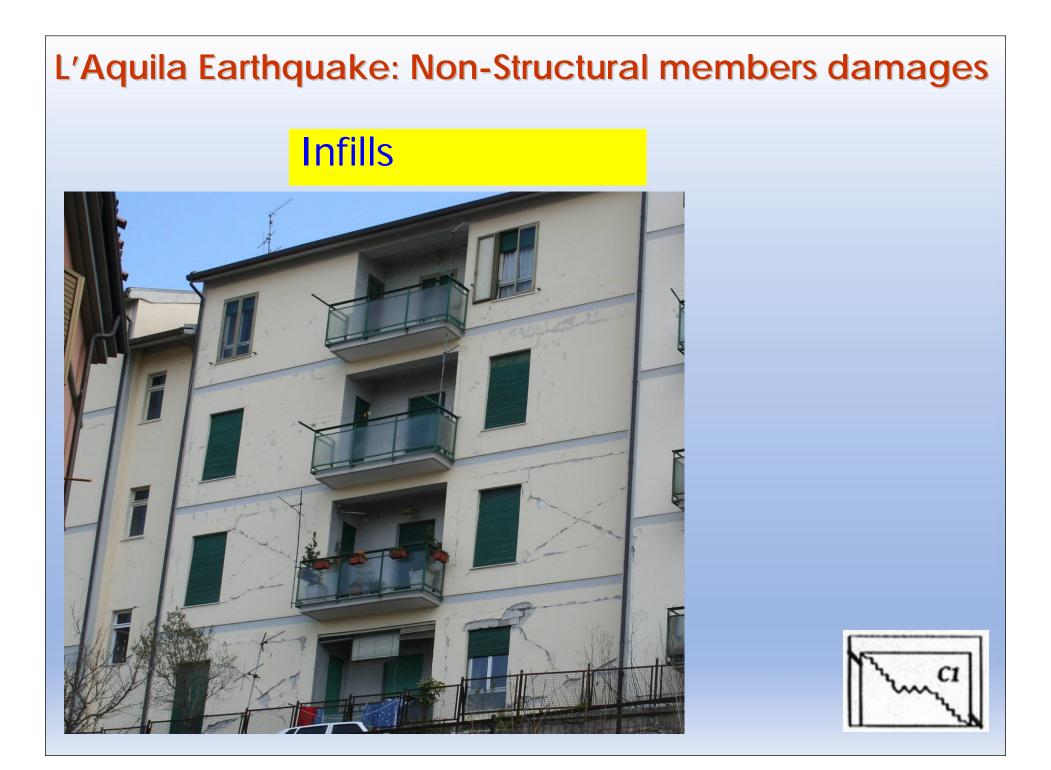
Failure due to diagonal crak: it could be followed by shear failure of rc frame



Failure due to the horizontal sliding along the partition's joint: in case of poor mortar or in case of poor bond between mortar and brciks



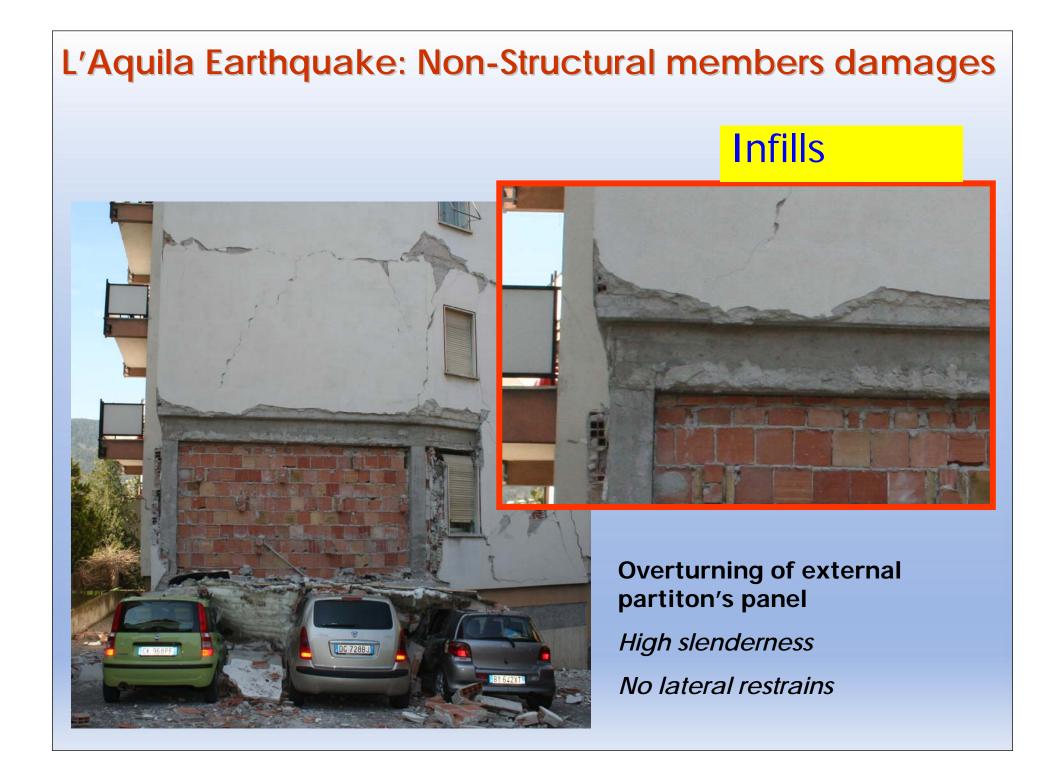
Failure due to partion's crushing near corners



Infills

Discontinuity due to openings Overturning of external partiton's panel

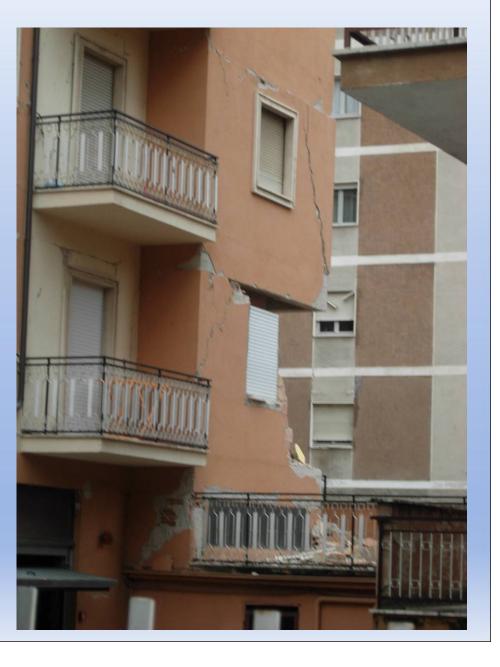




Infills

No lateral restrain.

Failure of external corner with damage of two sides.

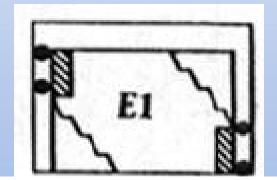


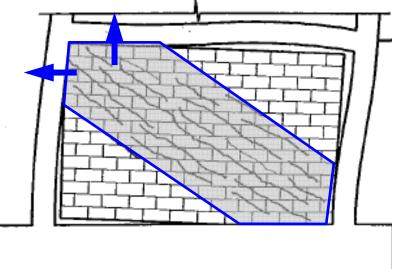
Infills

Typical partition crushing on corners due to the compressive stresses.

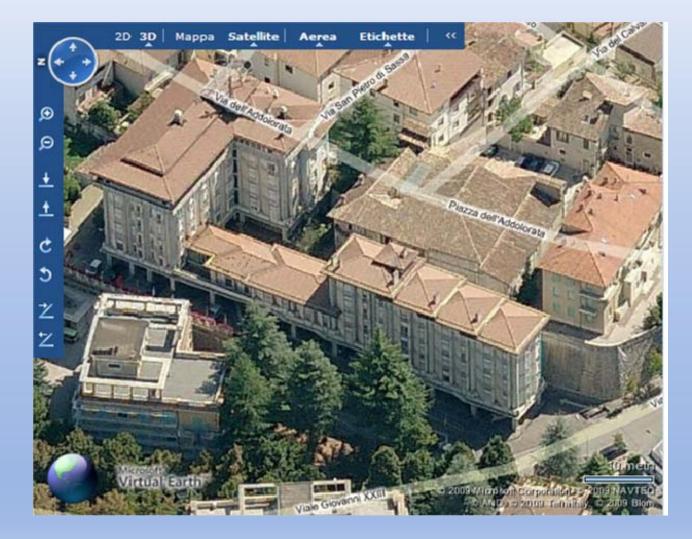
Diagonal flexural crack on the columns' end







L'Aquila Earthquake: Structural Collapses



Hotel Duca degli Abruzzi



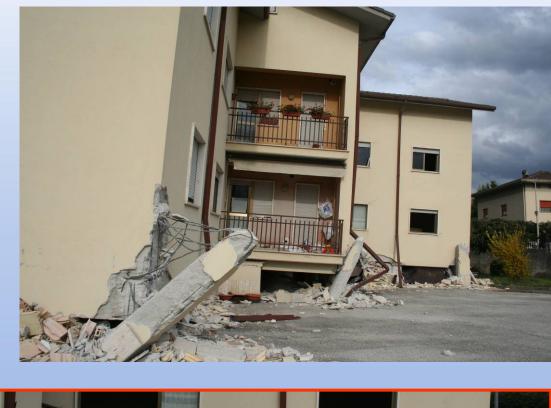
Hotel Duca degli Abruzzi



Hotel Duca degli Abruzzi



L'Aquila Earthquake: Structural collapses







The buildings of "Pettino"



REte dei Laboratori Universitari di Ingegneria Sismica

Damage to Masonry Buildings in Hystorical Center







Tempera: collapse of head board buildings

L'Aquila: lack of transversal wall at the upper level of the building

Damages and Vulnerability

Structural Behavior and building location in the masonry buildings' aggregate



Poggio Picenze: Local collapse due to a wall discontinuity



Villa Sant'Angelo: alteration of the original structural configuration: wooden tie,originally well connected to the external wall by means of an iron device, has been cutted during an heavy refurbishment



Tempera: the heavy roof caused the collapse of masonry walls

Villa Sant'Angelo: the r.c. roof implied the masonry walls collapse.





L'Aquila: at second floor it was inserted a r.c. stair.



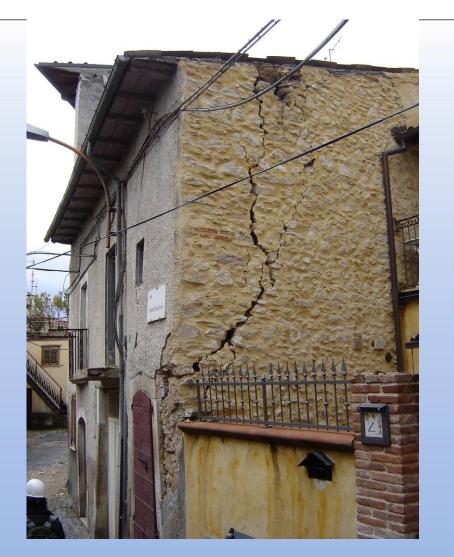
Paganica: insertion of a deep r.c. slab

Damages and Vulnerability

Floor slabs and Vaults



Castelnuovo: poor connection between masonry walls.



Paganica: overturning crack involving a portion of masonry wall.



Villa Sant'Angelo: in-plane shear cracks on the whole facade wall.

Sant'Eusanio Forconese: in-plane shear cracks on masonry walls





Structural Safety Assessment: In-situ inspections

THE BUILDINGS WERE RATED IN ORDER TO ORGANIZE THE REPAIR/RECONSTRUCTION PROCESS.

The "A" buildings were able to be inhabited again;

The "B" and "C" buildings required some light repairs before being reopened to habitants. The repairs on the "B" and "C" buildings were first priority. This is done in an effort to return people to their homes as quickly as possible. Many of the residents whose homes were considered "B" or "C" level have been temporary accomodated hotels and private homes accommodations because the repairs to their homes will not require as much time.

Structural Safety Assessment: In-situ inspections

THE BUILDINGS WERE RATED IN ORDER TO ORGANIZE THE REPAIR/RECONSTRUCTION PROCESS.

The "E" buildings are treated in a second stage of the reconstruction

Before the repairs were able to begin, many of the displaced people needed a more permanent shelter than a tent. The Civil Protection Department decided this mainly due to the climate in L'Aquila. The people needed to be out of the tent camps before winter hit. **The C.A.S.E. project came out of this need.**

The project consisted of the following goals: >Homes and Neighborhoods Available in Five to Six Months C.a.S.e. >Anti-seismic Security using an "Isolation" technique >High Level of Standard of Living >High-Tech, Self-Sufficient MEP Systems >Environmental Sustainability and Green Design

"Light Reconstruction" - Buildings "B","C"

Financial Contribution Allowed

Rating B: Temporarily unusable building (fully or partially) but will be fit for use with mainly non-structural interventions.
Rating C: Partially unusable building.

 ✓ Total refund of repair intervention costs + <u>local</u> <u>strengthening</u> of structural or non-structural members up to 150 €/mq.;



Local strengthening interventions:

- It is related to single structural members;
- no significant mass and stiffness variation;
- the local member's capacity increase should be evaluated;

• the analysis of the whole structure is not necessary.

"Heavy Reconstruction" - Buildings "E"

Financial Contribution Allowed

Rating E: Unusable building (structural damages).

- ✓ Total refund of repair intervention costs + <u>seismic rehabilitation</u> (up to 80%) up to 400-600 €/mq.
- ✓ In case of "E" buildings with a low level of structural damages it is also possible to perform only the <u>local</u> <u>strengthening</u> of structural or nonstructural members up to 250 €/mq.;





"Heavy Reconstruction" - Buildings "E"

Financial Contribution Allowed

Rating E: Unusable building (structural damages).

- ✓ Reconstruction or replacement in case of:
- 1) Totally collapsed buildings
- 2) <u>Masonry structures</u> partially collapsed (more than 25% in volume)
- 3) <u>Reinforced Concrete Structures</u>, average compressive cylindrical strength lower than 8 MPa
- 4) <u>Reinforced Concrete Structures</u>, more than 50% of storey's columns with a drift higher than 1.5%



Specific Reccomendation for masonry aggregates

RECONSTRUCTION PROCESS:

Reluis ACTIVITY

Several expert technicians in seismic risk (Ph.D, or master) perform the technical investigation both on:

"Light Reconstruction" - Buildings "B","C"

"Heavy Reconstruction" - Buildings "E"

RECONSTRUCTION PROCESS:

ReLUIS ACTIVITY ------

"Light Reconstruction" - Buildings "B","C"

WEBSITE: ReLUIS www.reluis.it

DESIGN KIT

GUIDELINES AND DESIGN EXAMPLES GUIDELINES FOR REPAIR AND LOCAL STRENGTHENING OF STRUCTURAL AND NON STRUCTURAL MEMBER

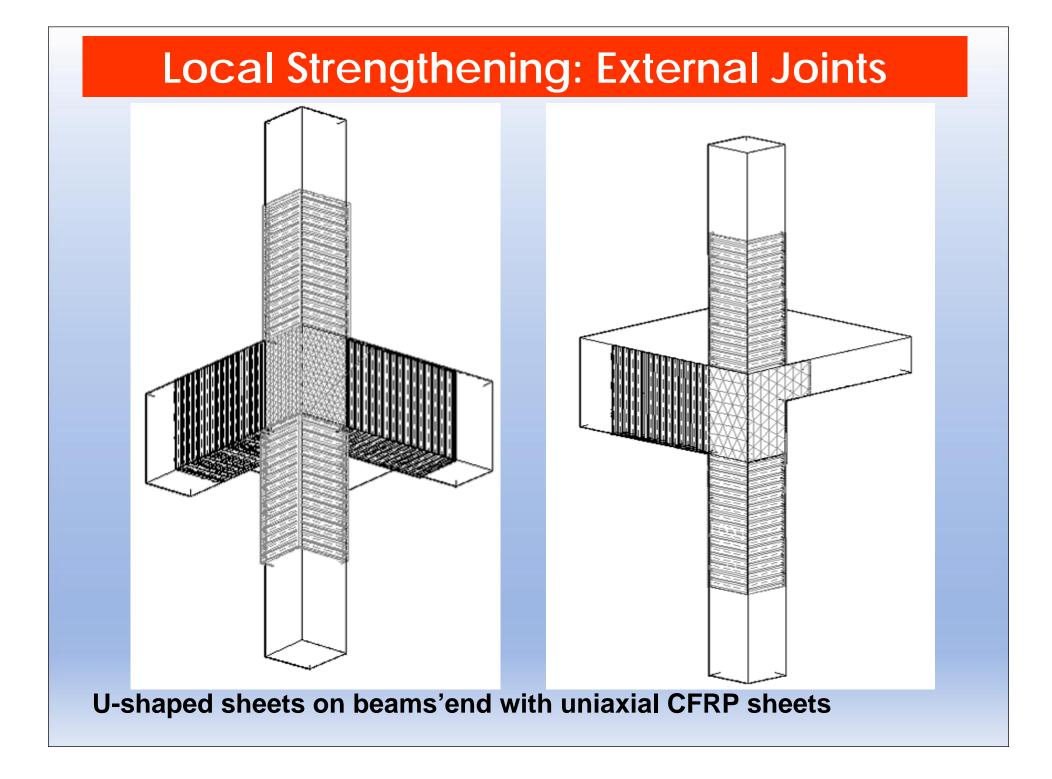


1. Repair of damaged non structural members (A.1);

2. Local repair of structural members (A.4);

3. Interventions to avoid overturning of internal or external partitions (B.1);

4. Local strengthening interventions on reinforced concrete or masonry structures according to seismic codes (8.4.3, DM 14.01.08 and Circolare n. 617, 2 febbraio 2009 (B.2).



Local Strengthening: External Joints





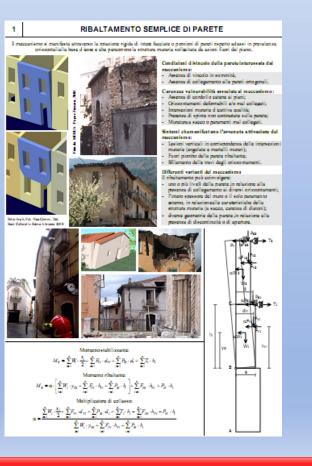
Plaster removal along the partition's perimeter.

WEBSITE ReLUIS www.reluis.it

DESIGN KIT

GUIDELINES AND DESIGN EXAMPLES

LOCAL COLLAPSE MECHANISMS IN EXISTING MASONRY STRUCTURES



1. Synthetic description of collapse mechanisms;

- 2. Sketch and photos of real collapses;
- 3. Constrains of masonry walls;
- 4. Vulnerability due to the mechanism.

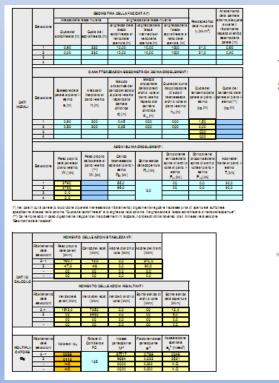
reluis

WEBSITE ReLUIS www.reluis.it

DESIGN KIT

GUIDELINES AND DESIGN EXAMPLES

DESIGN EXAMPLE OF LOCAL STRENGTHENING: CHAINS







To avoid the out of plane overturning of masonry walls.

1. Verification at Life Safety limit state with linear analysis ;

2. Verification at Life Safety limit state with non linear analysis;

3. Constructions' details .

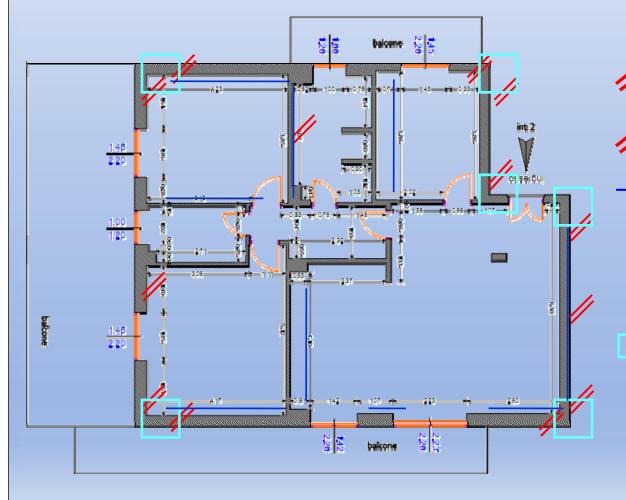
CHECK LIST: DOCUMENTATION

STRCTURE DESCRIPTION AND DAMAGES' REPORT



CHECK LIST: DOCUMENTATION

DESCRIPTION AND DESIGN OF INTERVENTIONS



REPAIR INTERVENTION

F/03-005 Stuccatura e rinzeppatura di lesioni profonde su murature in laterizio forato.

NP 84 Ripresa di lesione con funzione antiribaltamento mediante apposizione di rete in fibra di vetro su strato di malta bicomponente ad elevata duttilità .

NP 05 RASATURA ARMATA PER INTERNO. Rasatura armata per recupero di parti interne interessate da lesioni di intonaco.

LOCAL STRENGTHENING INTERVENTIONS (up to 150 €m²)

F/ 1-45 Rinforzo strutturale su elementi inflessi e pressoinflessi mediante l'applicazione di nastri in fibra di carbonio.

"Ligth" Reconstruction – BUILDINGS "B" – "C"

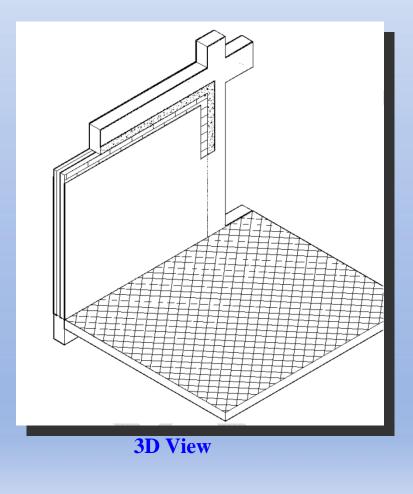
Damages to non structural



- 1. External panel of partition collapse;
- 2. Partition overturning:
- 3. Cracks on partitions.

"Ligth" Reconstruction – BUILDINGS "B" – "C"

Partitions' Anti-Overturning



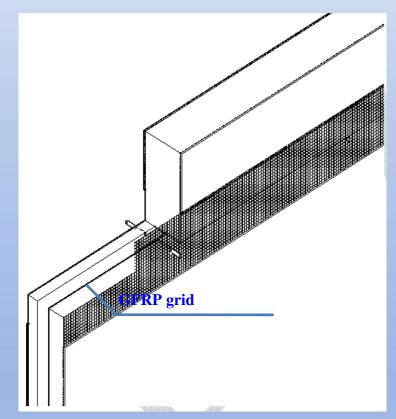
1. Plaster removal along the partition's perimeter;



Section

"Ligth" Reconstruction – BUILDINGS "B" – "C"

Partitions' Anti-Overturning



3D View

2. Hole in the partition;

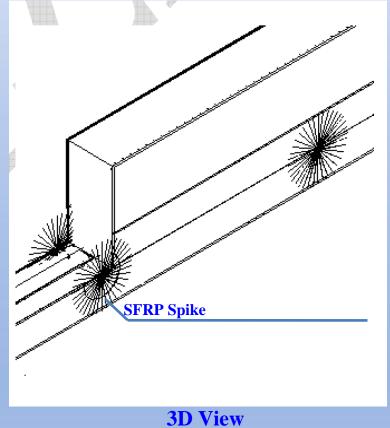
3. Installation of a first layer of cementitious mortar and GFRP alcali-resistant grid;



Section

"Ligth" Reconstruction – BUILDINGS "B" – "C"

Partitions' Anti-Overturning



4. SFRP spikes;

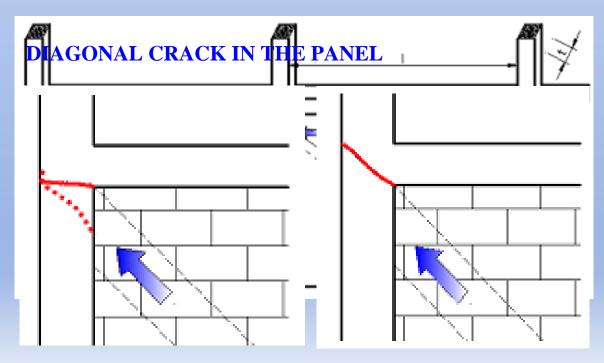


Section

"Ligth" Reconstruction – BUILDINGS "B" – "C"

Structural Damages (Repair or Local Interventions)

JOINT FAILURE: Cracks due to the actions provided by partition. PSEUDO-HORIZONTAL CRACK at column-joint panel interface.

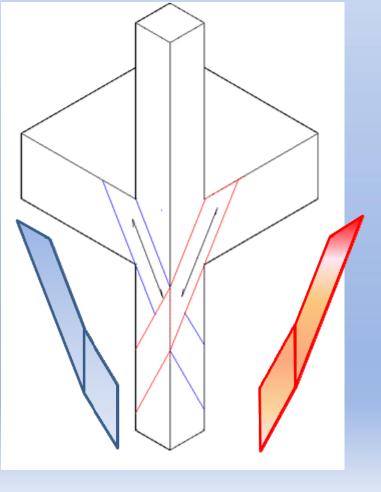




"Ligth" Reconstruction – BUILDINGS "B" – "C"

Structural Damages (Repair or Local Interventions)

Strengthening of external Joints

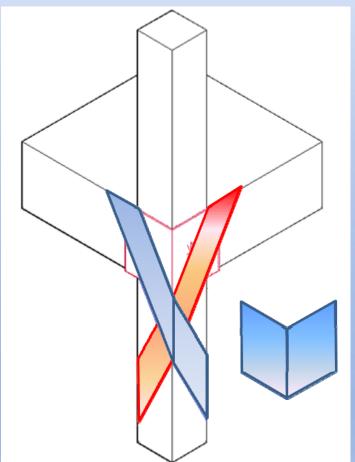


1. DIAGONAL UNIAXIAL STEEL SHEETS designed to resist to partitions' actions .



"Ligth" Reconstruction – BUILDINGS "B" – "C"

Structural Damages (Repair or Local Interventions)

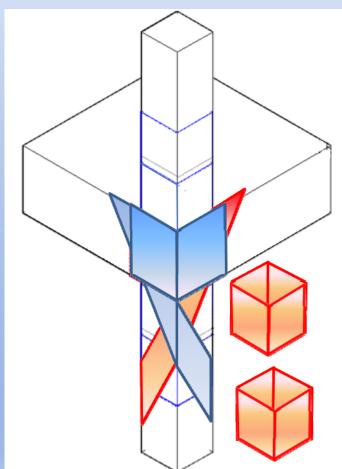


2.QUADRAXIAL CFRP SHEETON ON THE JOINT PANEL: Shear strength increase.



"Ligth" Reconstruction – BUILDINGS "B" – "C"

Structural Damages (Repair or Local Interventions)

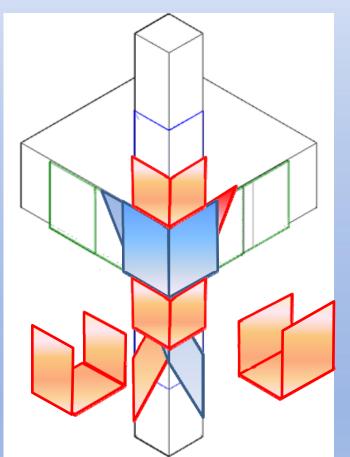


3. COLUMNS' ENDS CONFINEMENT BY USING CRP UNIAXIAL SHEETS: Concrete ultimate strain increase **e** and thus curvature and rotational capacity increase



"Ligth" Reconstruction – BUILDINGS "B" – "C"

Structural Damages (Repair or Local Interventions)



4. U-SHAPED UNINAXIAL CFRP SHEETS : Shear strength increase of beams'ends



"Ligth" Reconstruction – BUILDINGS "B" – "C"

MASONRY STRUCTURES Out of plane mechanisms



DAMAGES OVERTURNING

"Ligth" Reconstruction – BUILDINGS "B" – "C"

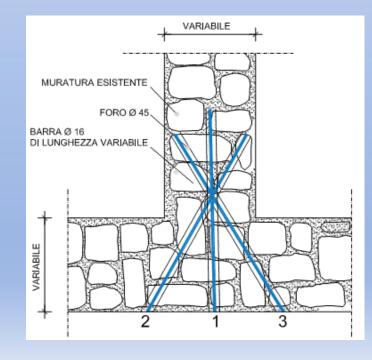
MASONRY STRUCTURES

Repair and Strengthening Interventions

TO AVOID OVERTURNING:

CORNER JOINTS STRENGTHENING





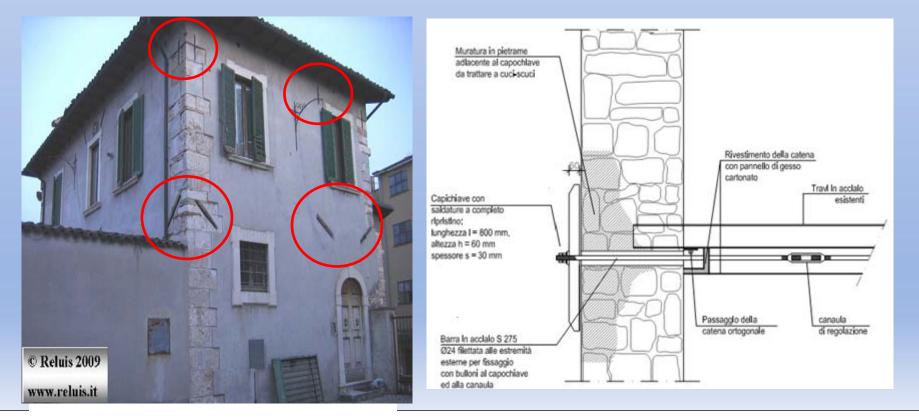
"Ligth" Reconstruction – BUILDINGS "B" – "C"

MASONRY STRUCTURES

Repair and Strengthening Interventions

TO AVOID OVERTURNING:

CHAINS



"Ligth" Reconstruction – BUILDINGS "B" – "C"

MASONRY STRUCTURES Repair and Strengthening Interventions

TO AVOID OVERTURNING:

onnettori a piolo per ad si sti acciato cile, bil 40 com

> Peatra in acciaio excione rettangolare

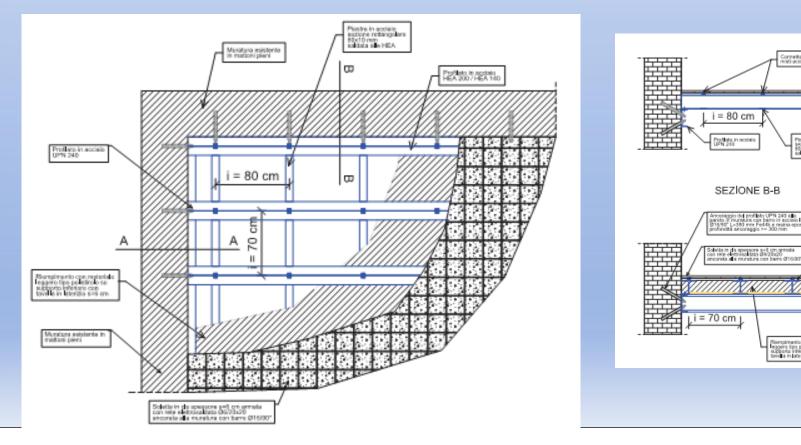
60x10 mm saldata alle HEA

196

Profilato in acela HEA 200

> rollato in acciaio PN 240

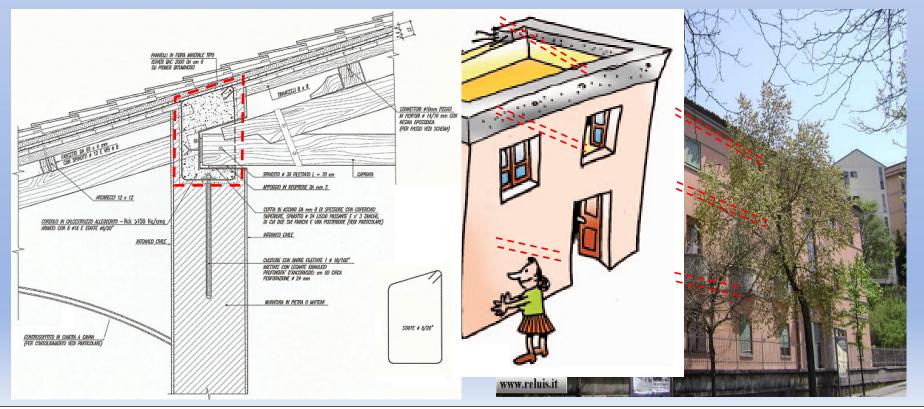
CONNECTION BETWEEN HORIZONTAL SLAB AND PERIMETER MASONRY WALLS



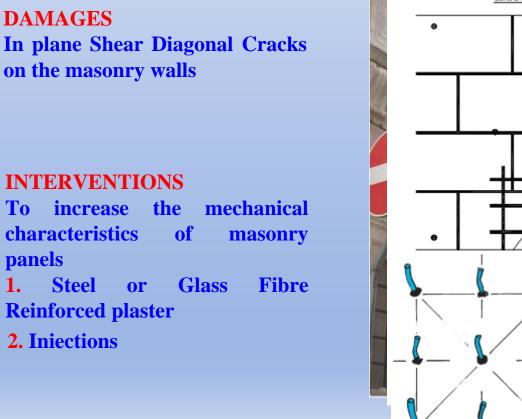
"Ligth" Reconstruction – BUILDINGS "B" – "C"

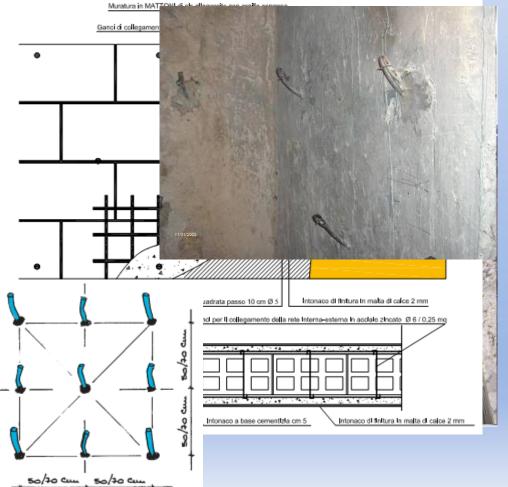
MASONRY STRUCTURES Repair and Strengthening Interventions TO AVOID OVERTURNING:

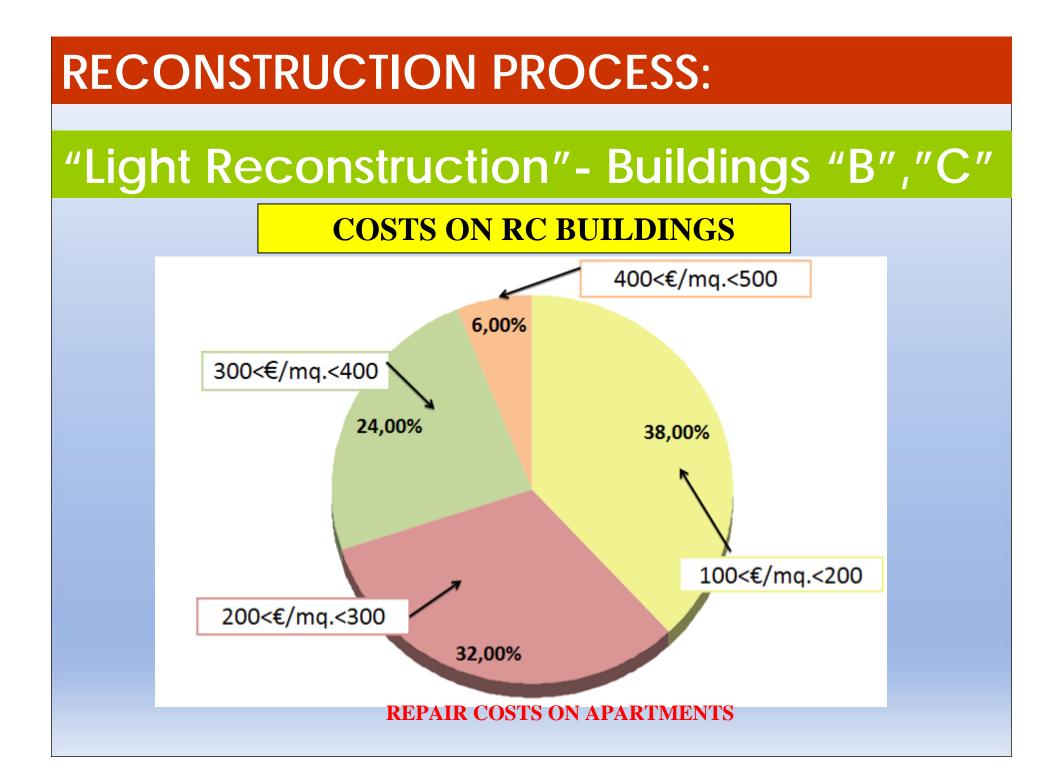
RC TIE BEAMS

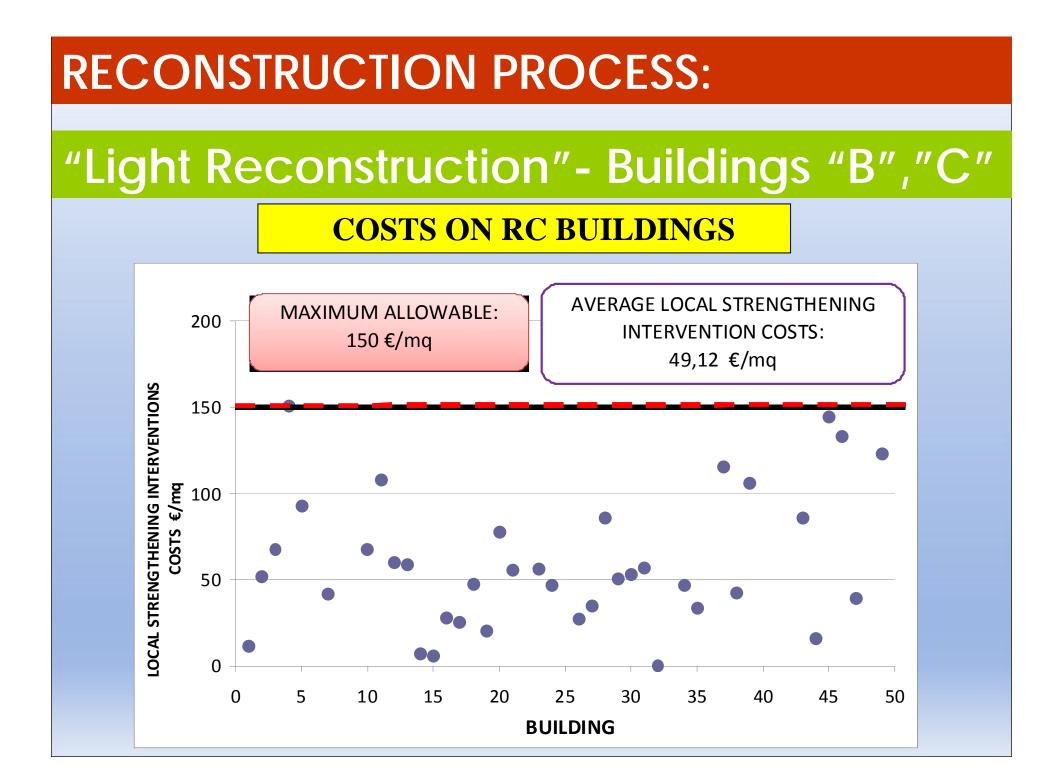


"Ligth" Reconstruction – BUILDINGS "B" – "C" MASONRY STRUCTURES In plane mechanisms





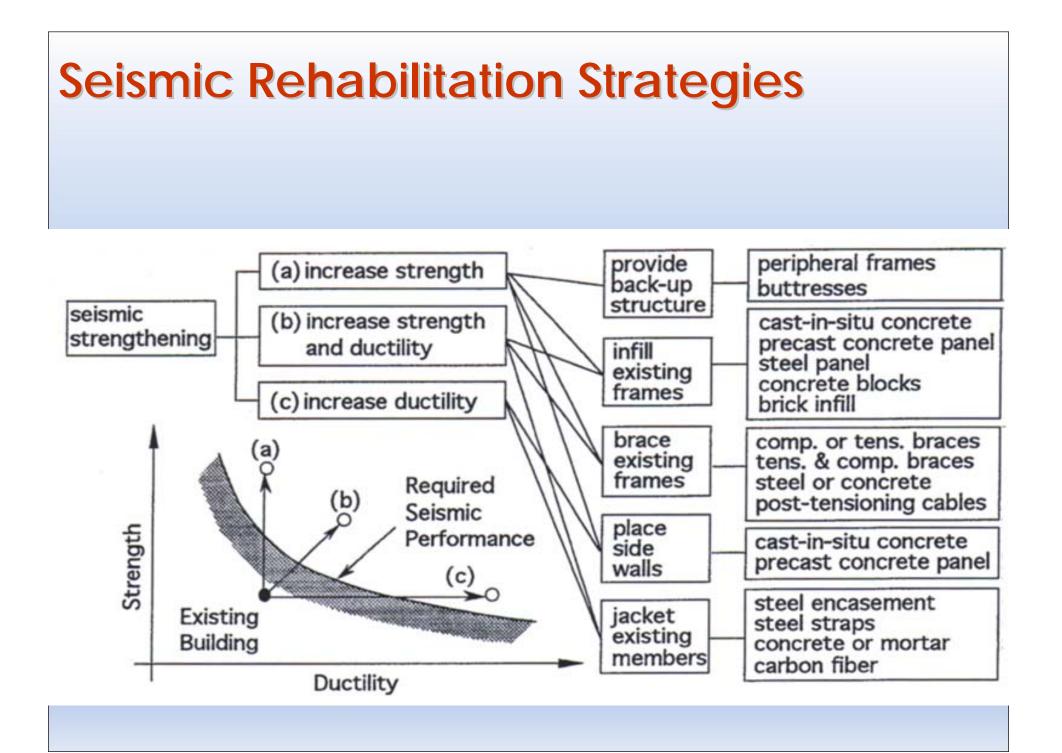




RECONSTRUCTION PROCESS:

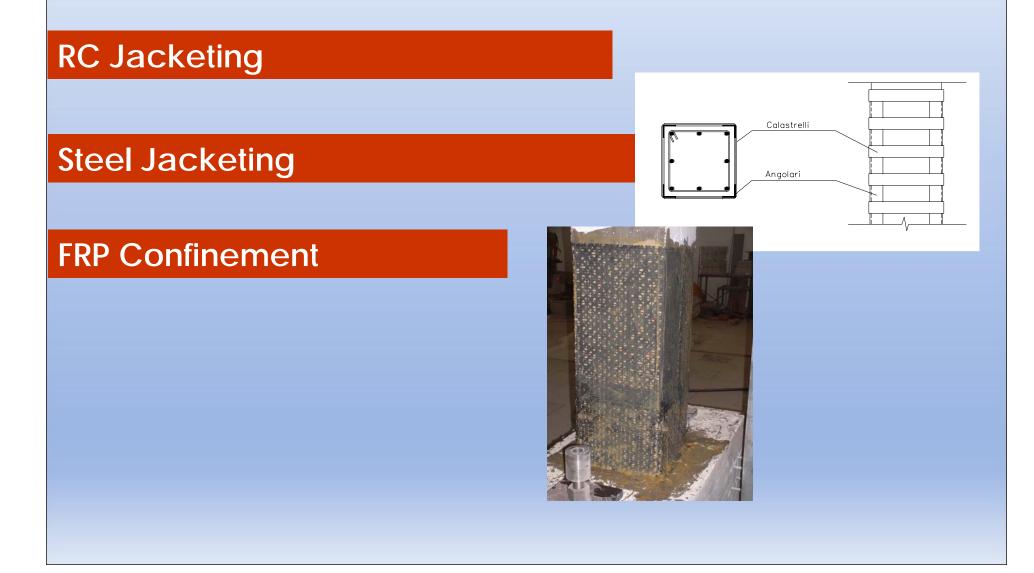
ReLUIS ACTIVITY

"Heavy Reconstruction" - Buildings "E"



Seismic Rehabilitation Strategies

Local Strengthening Techniques



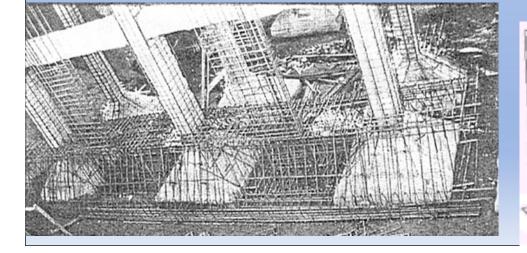
Seismic Rehabilitation Strategies Global Strengthening Techniques

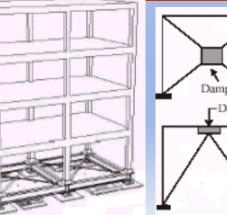
Steel bracing

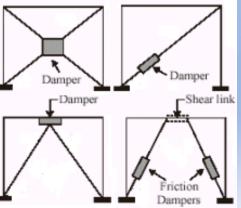


RC Shear Walls

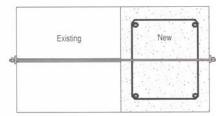
Base Isolators and dissipative bracing system



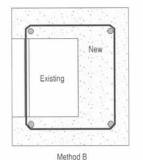


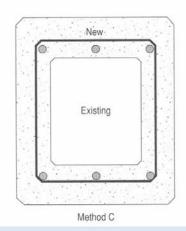


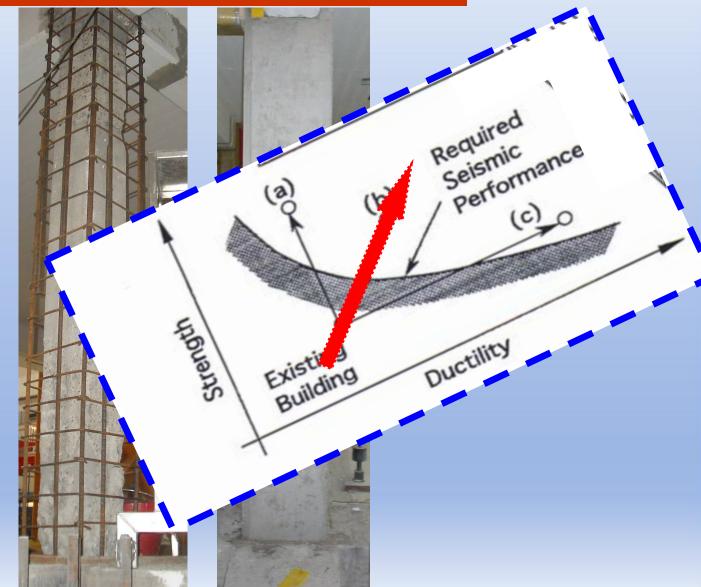
Local Strengthening Techniques RC Jacketing







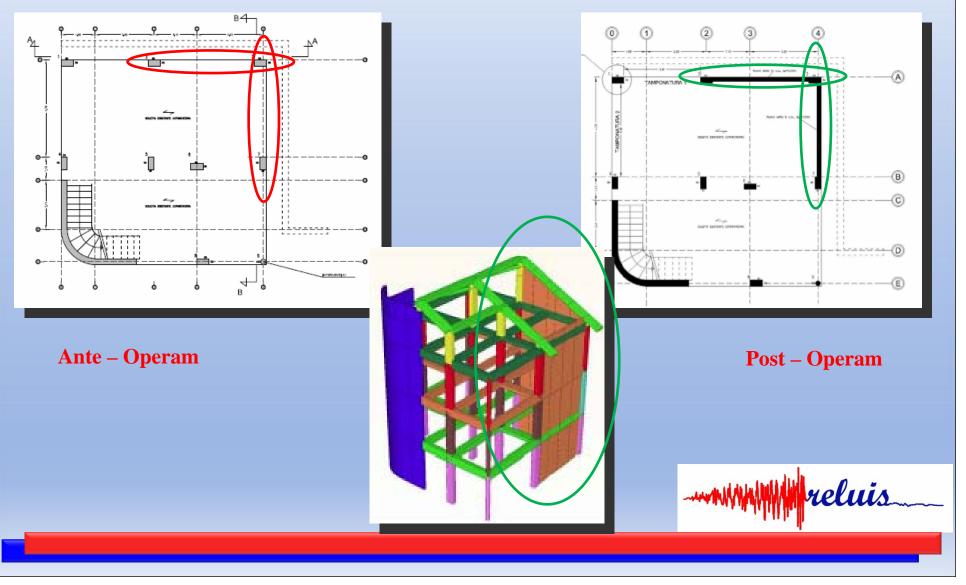




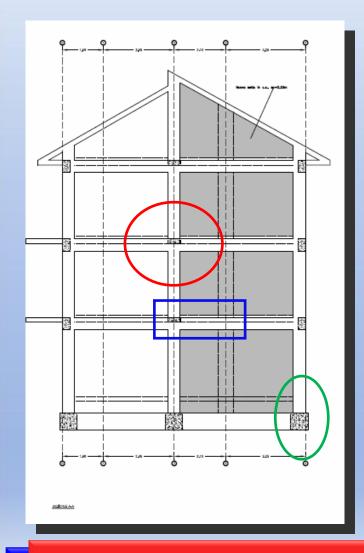
Local Strengthening Techniques FRP Confinament 50 repaired 45 Required Seismic Performance 40 35 (kN) (0) Strength Ductility Existing Building 120 140 160 100

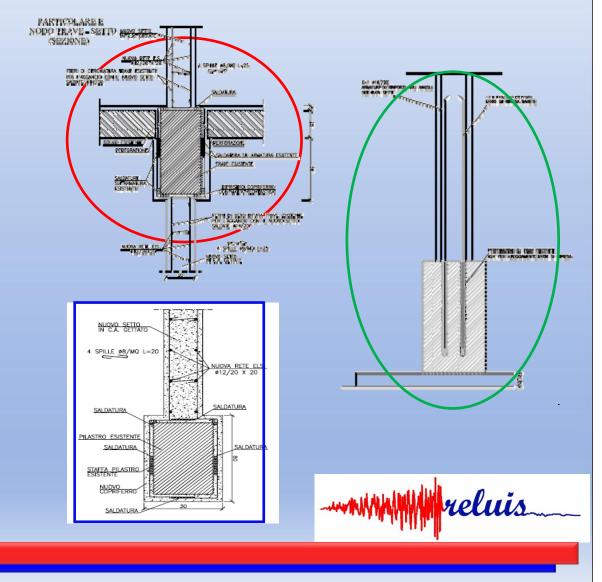


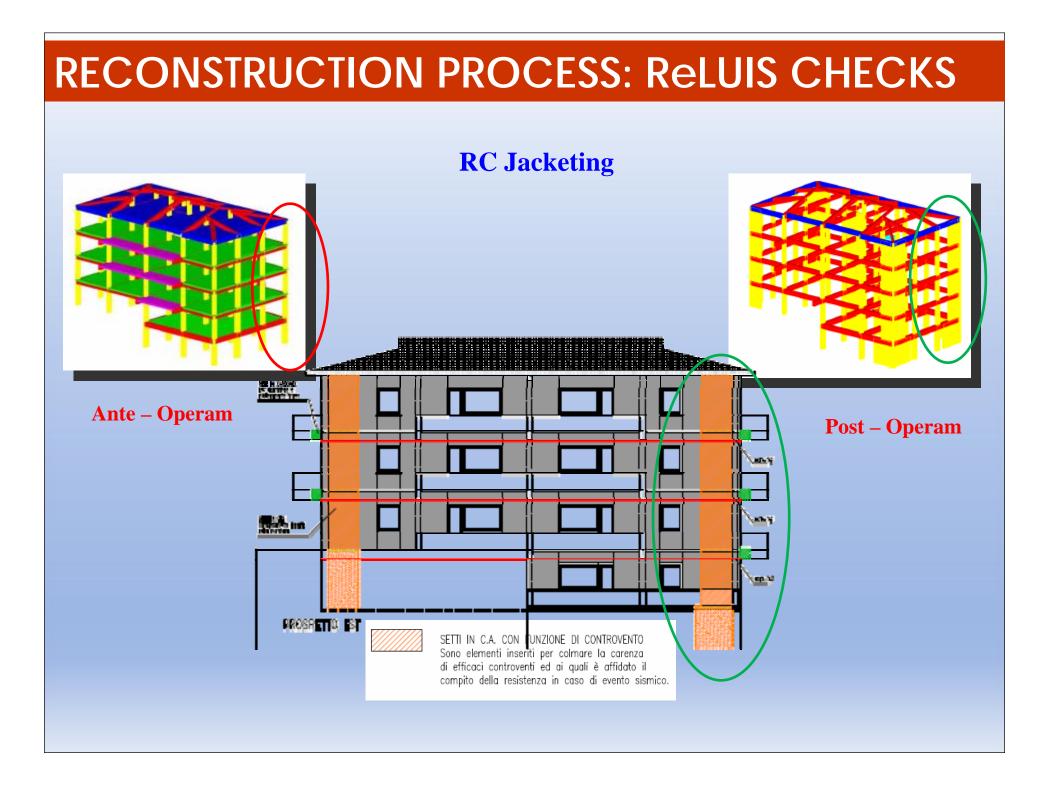
RC Shear Walls



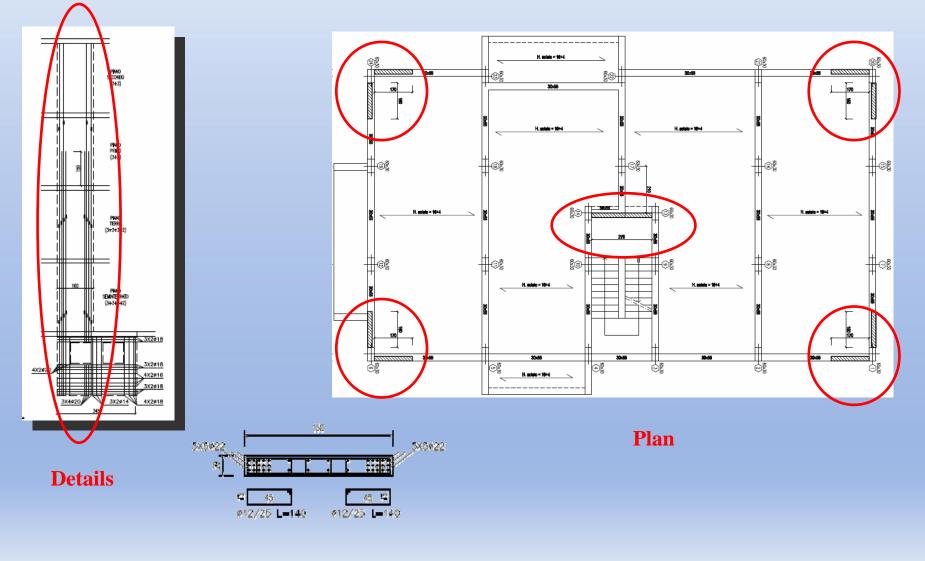
RC Shear Walls



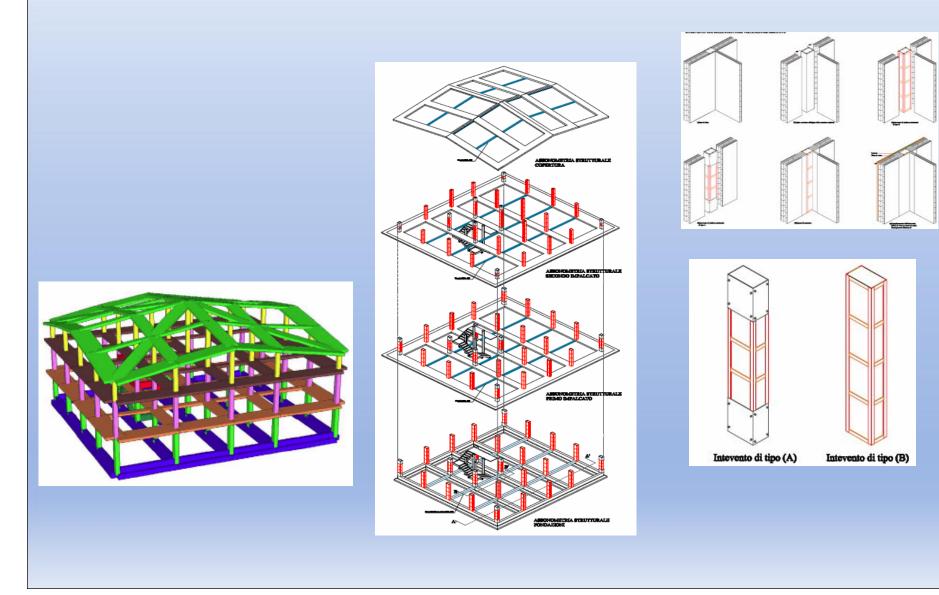




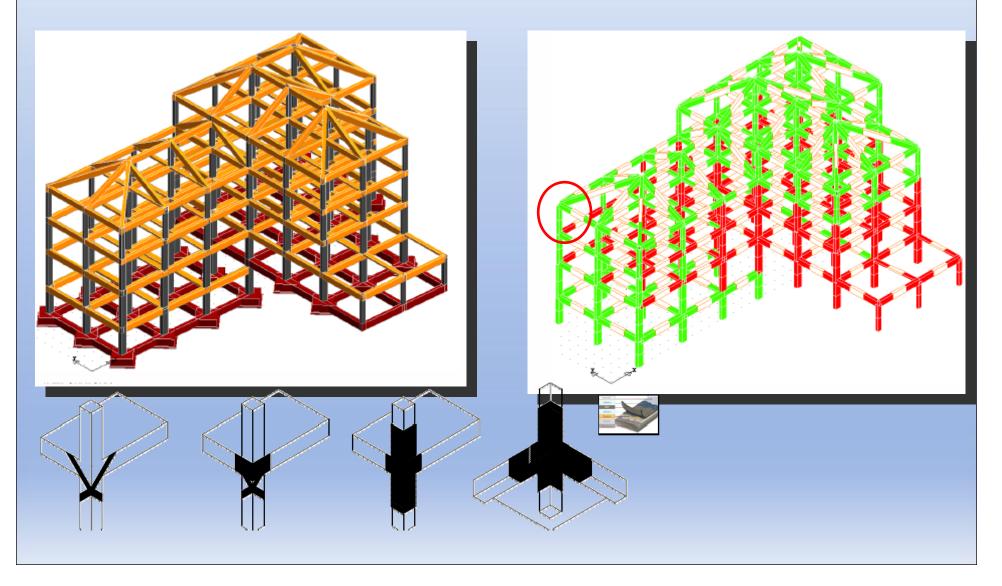
RC Jacketing



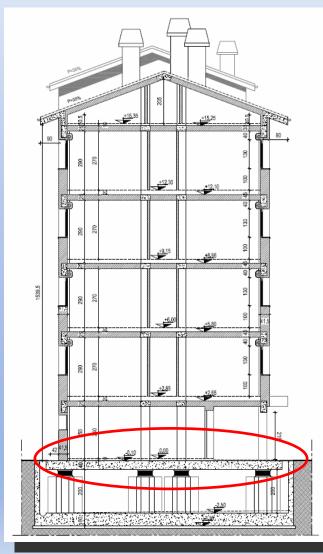
Steel Jacketing

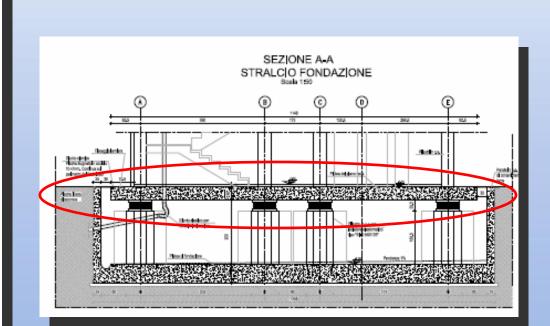


FRP Strengthening - Global analysis and details

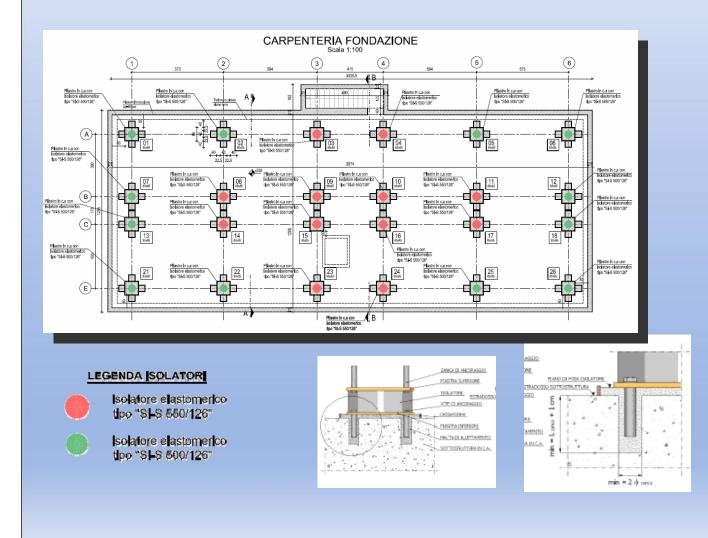


Base Isolation – Case A





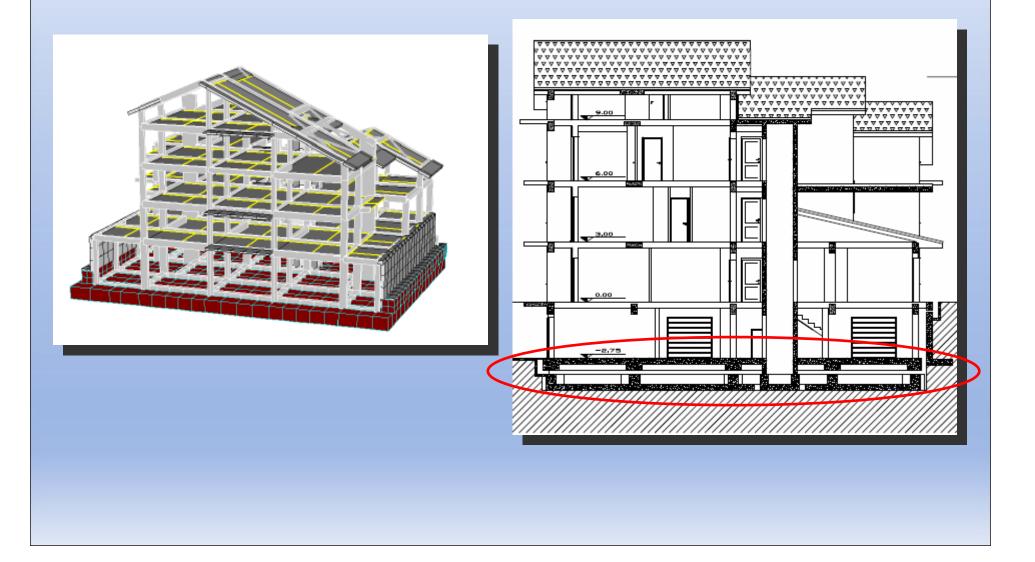
Base Isolation – Case A

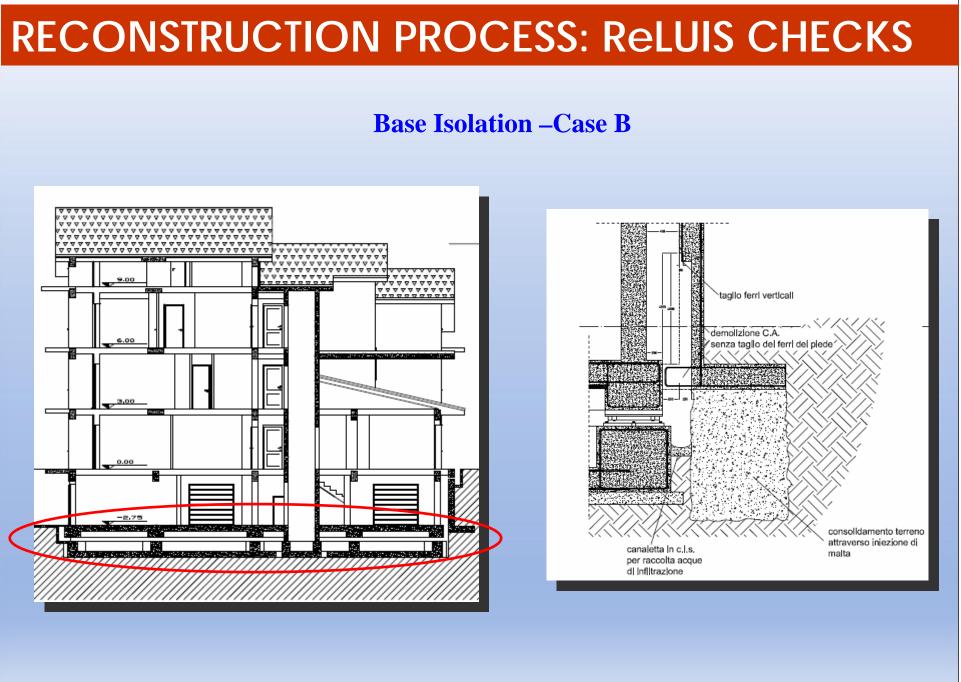




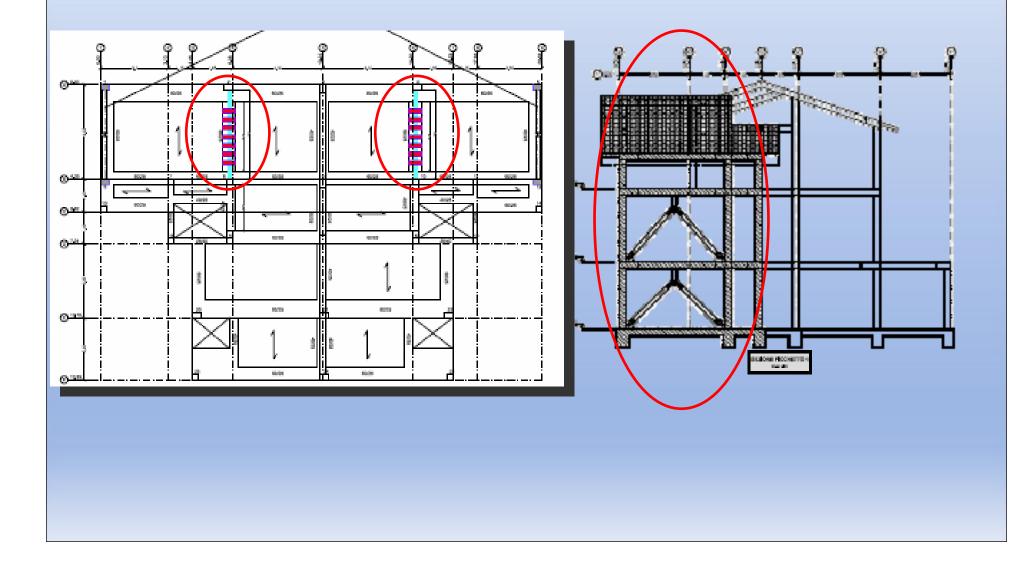


Base Isolation – Case B

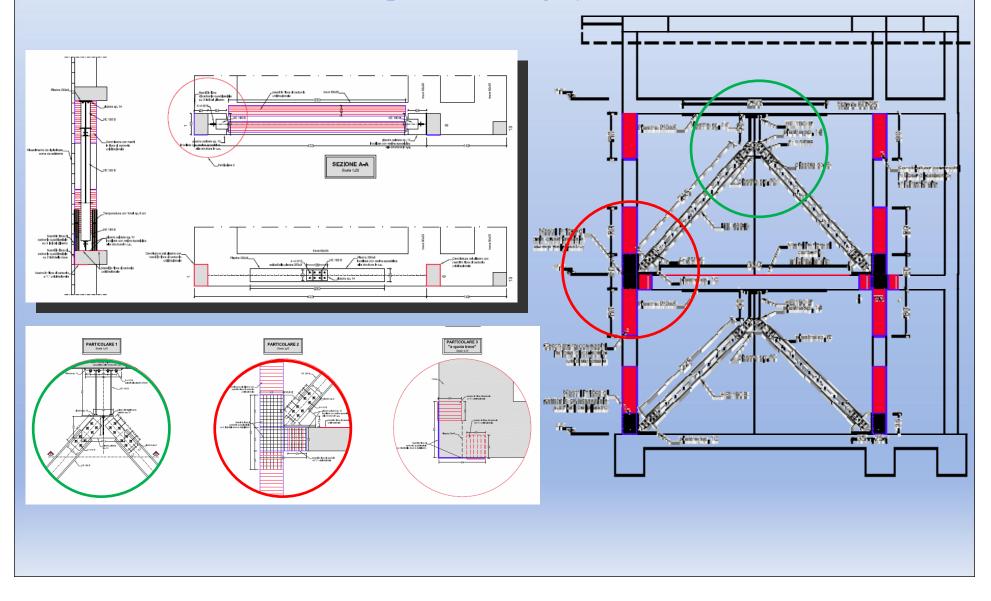




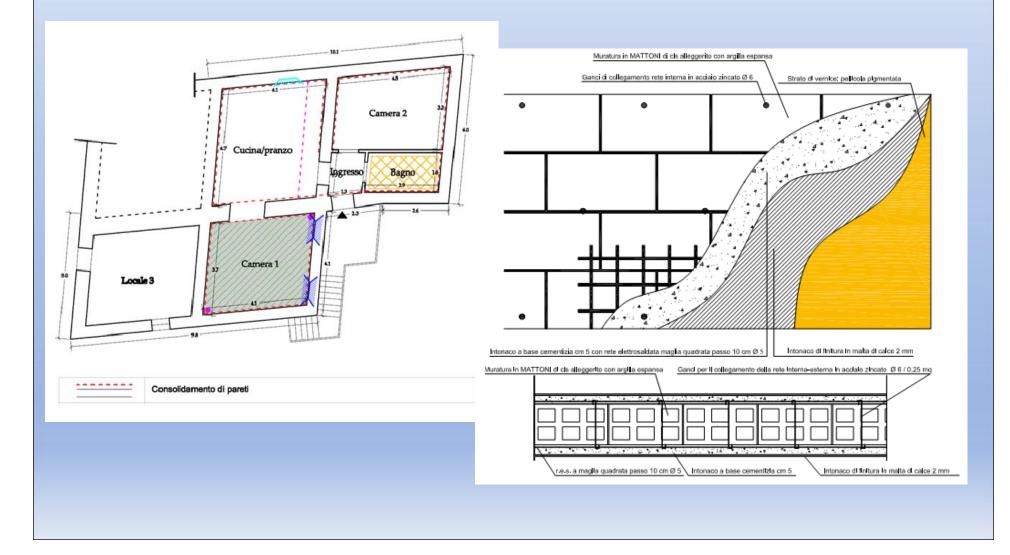
Dissipative Bracing Systems



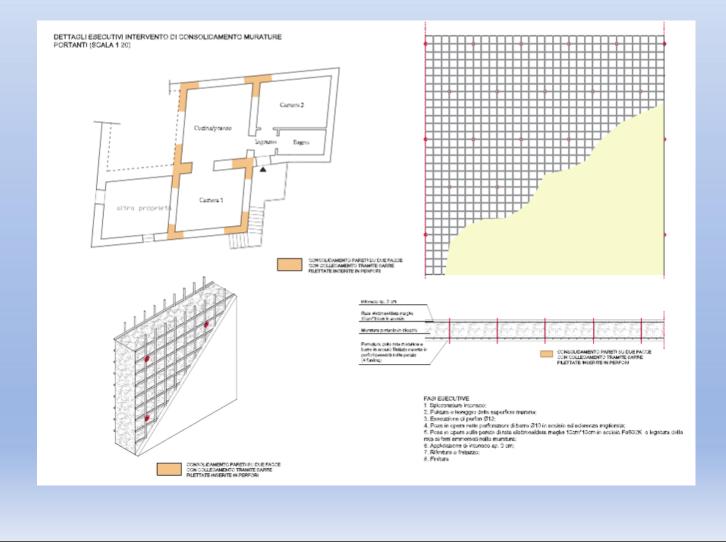
Dissipative Bracing Systems



Steel Reinforced plaster

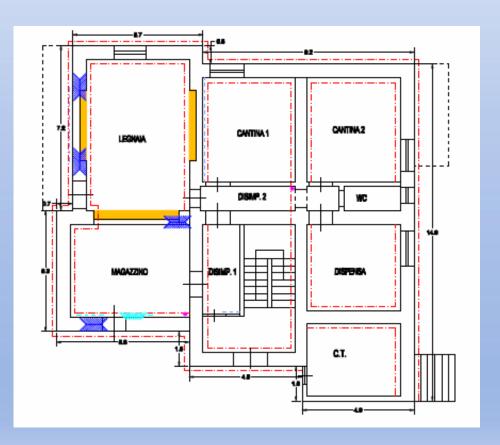


Localized Steel or Glass Fibre Reinforced plaster



Diffused Steel or Glass Fibre Reinforced plaster

Global Analysis is needed



RESTARTING FROM L'AQUILA



REte dei Laboratori Universitari di Ingegneria Sismica

Summary of ReLUIS Checks

<u>3152</u> PROJECTS INVESTIGATED OF





REte dei Laboratori Universitari di Ingegneria Sismica

The reconstruction of hystoric centre of L'Aquila



"PIAZZA della PREFETTURA":

A PILOT PROJECT

METHODOLOGY OF STUDY

- 1. ARCHITECTURAL SURVEY
- 2. STUDY OF MASONRY BUILDINGS' AGGREGATE AND ARCHITECTURAL CHARACTERISTICS
- 3. STRUCTURAL SURVEY
- 4. DAMAGE AND CRACKS ANALYSIS
- 5. IN SITU TESTS
- 6. SEISMIC ANALYSIS
- 7. RETROFIT INTERVENTIONS
- 8. STRUCTURAL CAPACITY INCREASE

DEFINITION OF GUIDELINES FOR THE SURVEY, ANALYSIS AND RETROFIT INTERVENTIONS ON MASONRY BUILDINGS' AGGREGATE IN SEISMIC ZONES





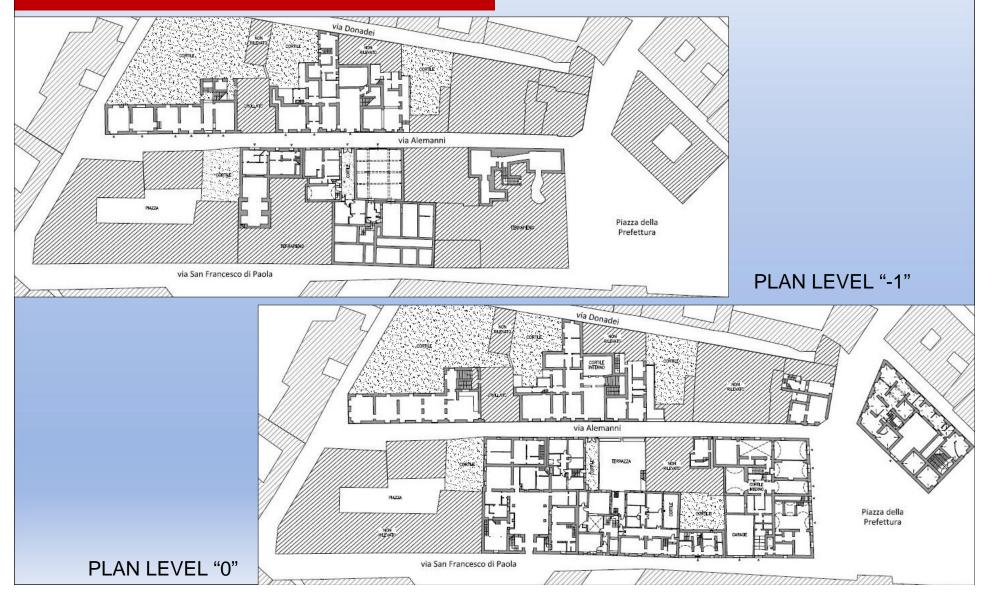
ARCHITECTURAL SURVEY

"BLOCK "PIAZZA della PREFETTURA"



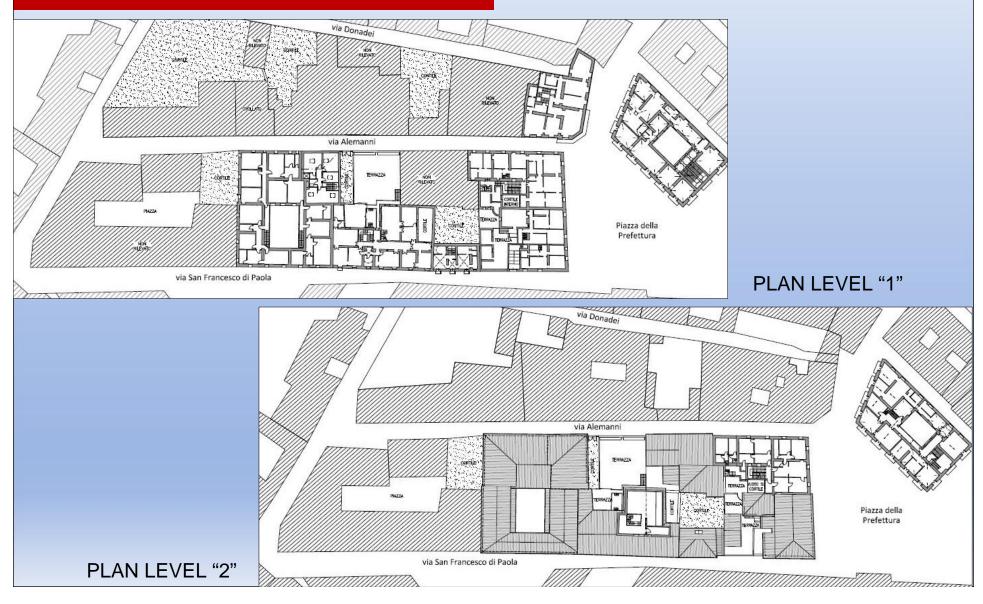
ARCHITECTURAL SURVEY

BLOCK "PIAZZA della PREFETTURA"



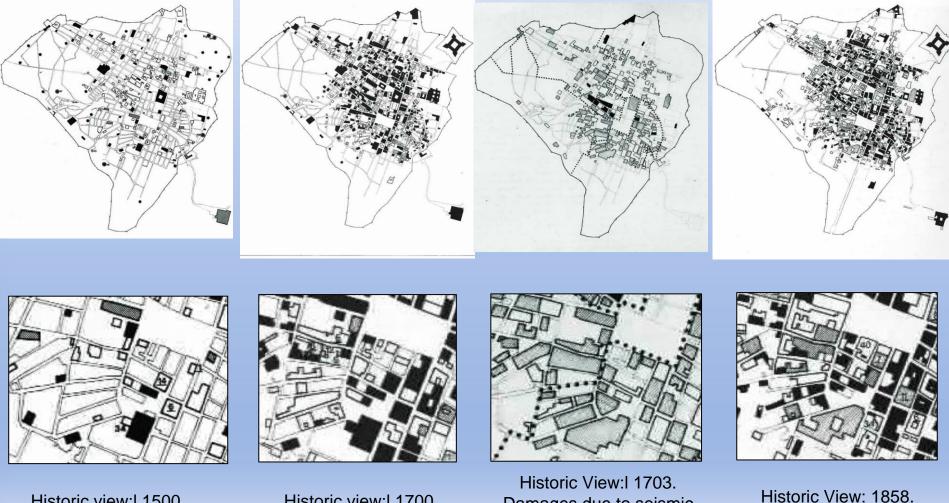
ARCHITECTURAL SURVEY

BLOCK "PIAZZA della PREFETTURA"



MASONRY BUILDINGS' AGGREGATE

BLOCK "PIAZZA della PREFETTURA"



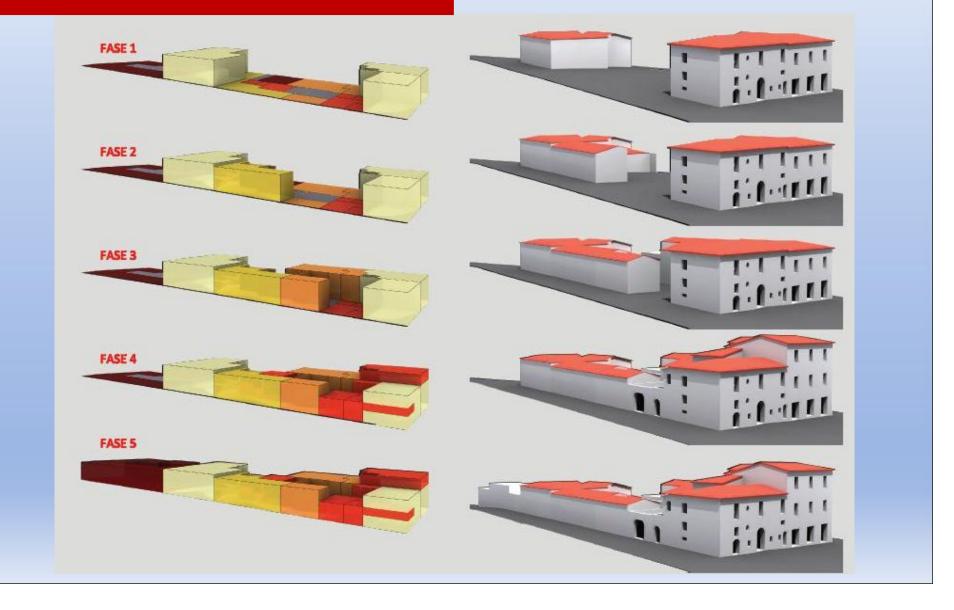
Historic view: 1500.

Historic view: 1700.

Damages due to seismic event

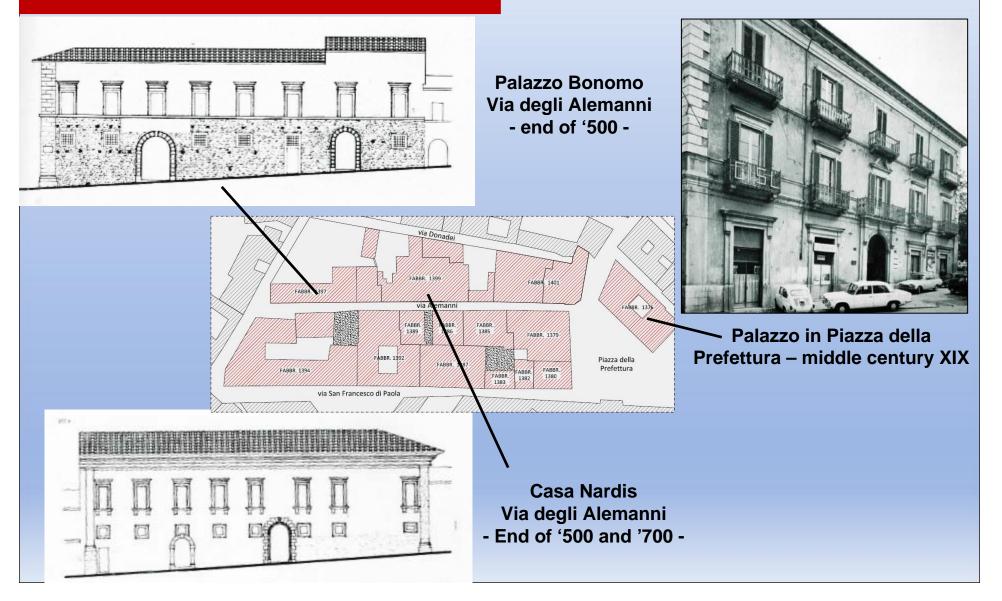
MASONRY BUILDINGS' AGGREGATE

BLOCK "PIAZZA della PREFETTURA"

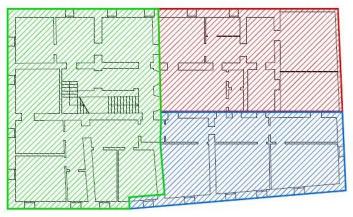


MASONRY BUILDINGS' AGGREGATE

BLOCK "PIAZZA della PREFETTURA"

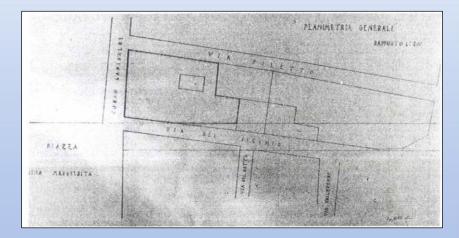


ARCHITECTURAL SURVEY AND BLOCK STUDY

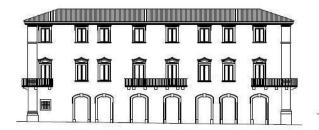




"BLOCK via VITTORIO EMANUELE II"



Designer Ing. Lisi (1930) extension "via Vittorio Emanuele II"





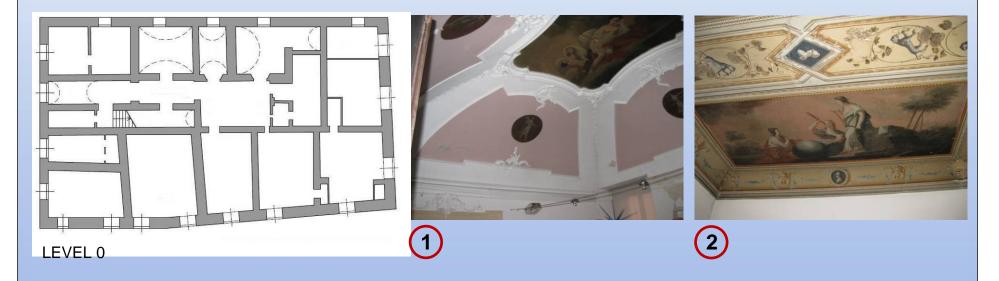


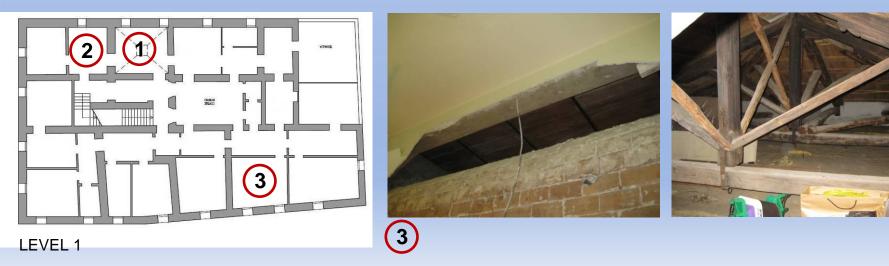


PROSPETTI

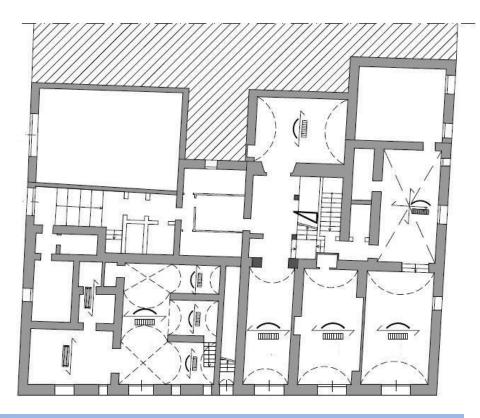
ARCHITECTURAL SURVEY AND BLOCK STUDY

"BLOCK via VITTORIO EMANUELE II"

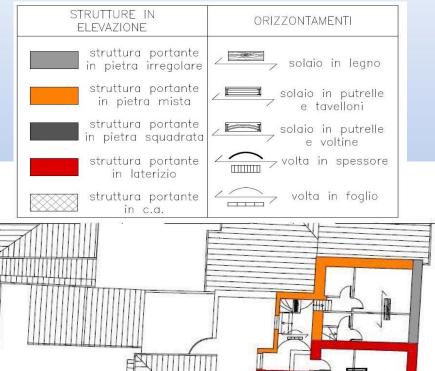


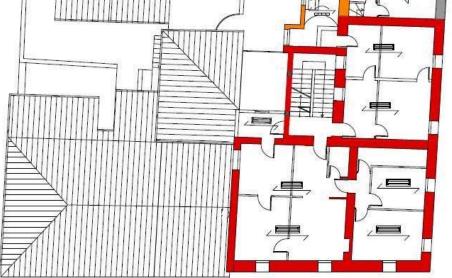




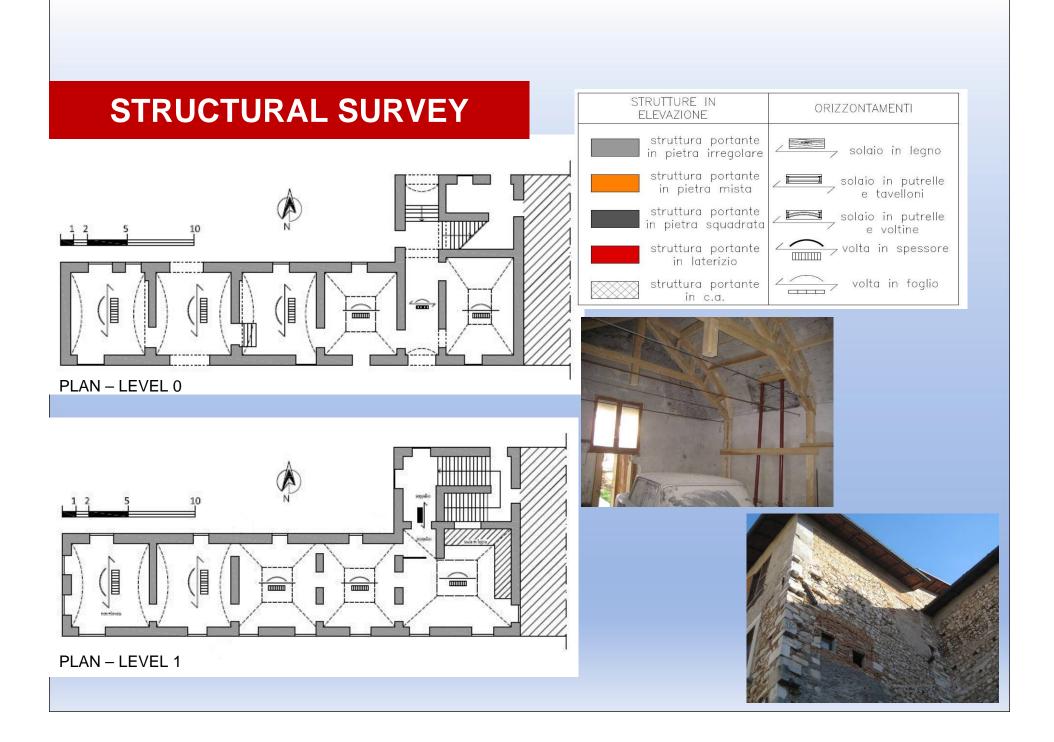


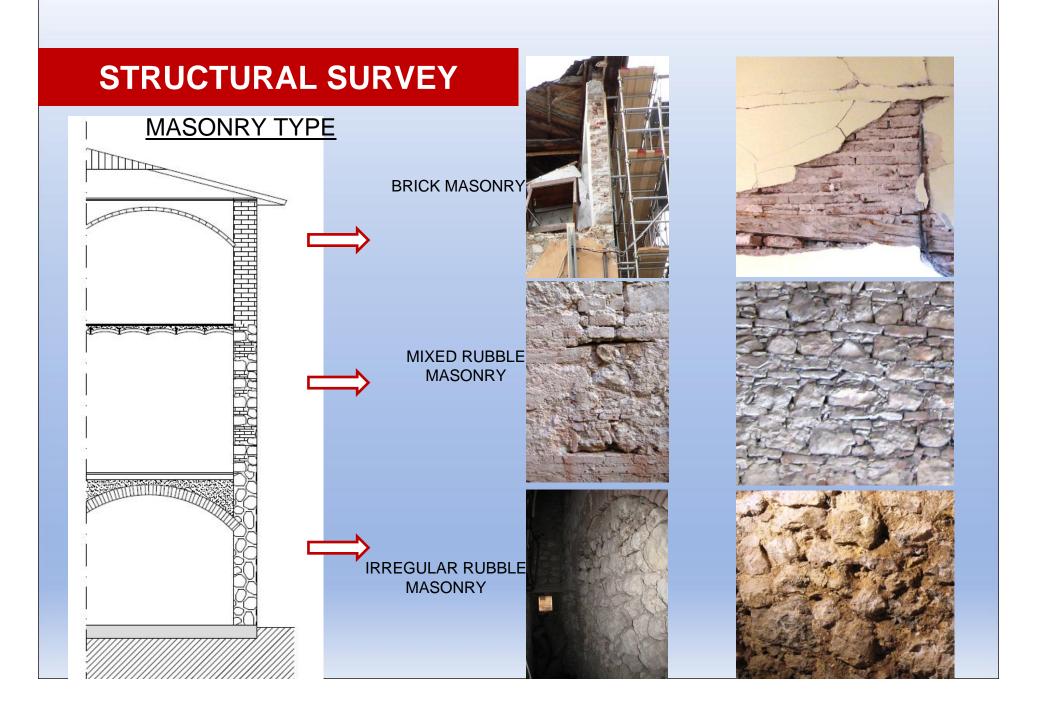
PIANTA PIANO TERRA





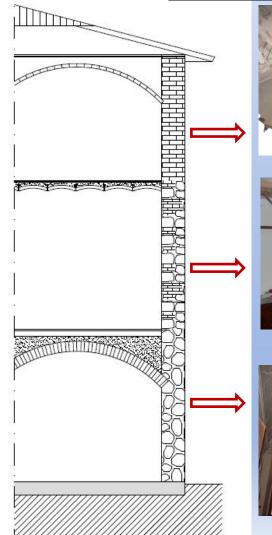
PIANTA PIANO SECONDO





STRUCTURAL SURVEY

FLOOR SLAB





CLOISTER VAULT



THIN VAULT



MIXED BEAM-VAULT FLOOR SLAB



BARREL VAULT



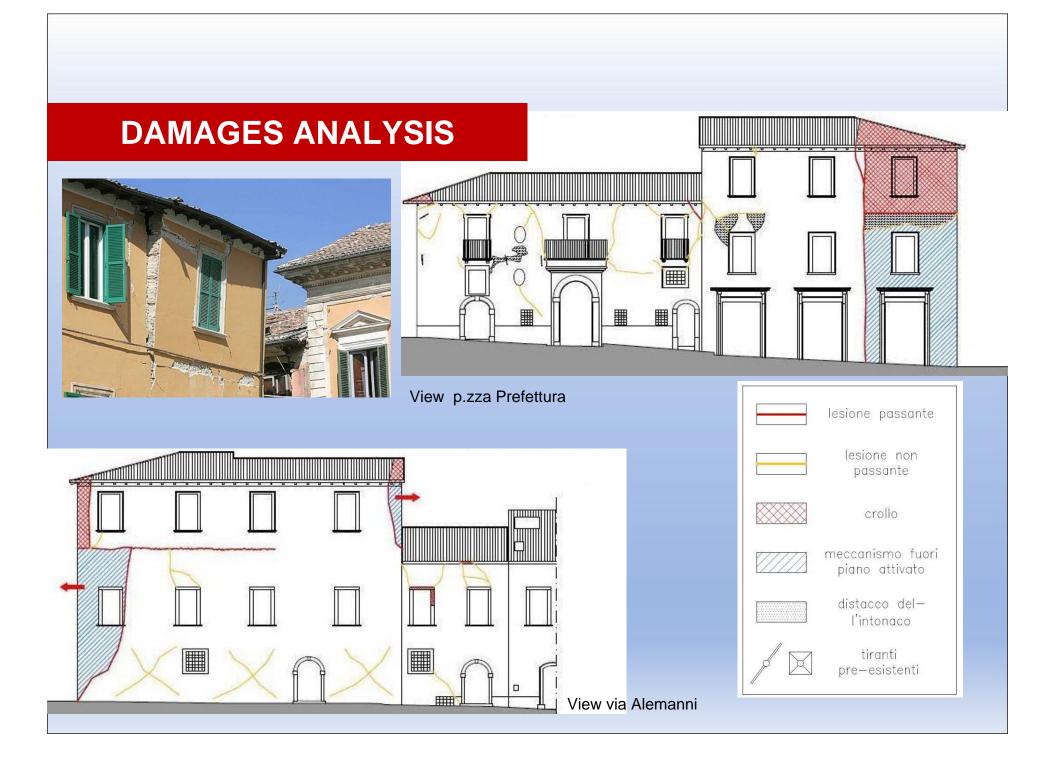
THIN VAULT

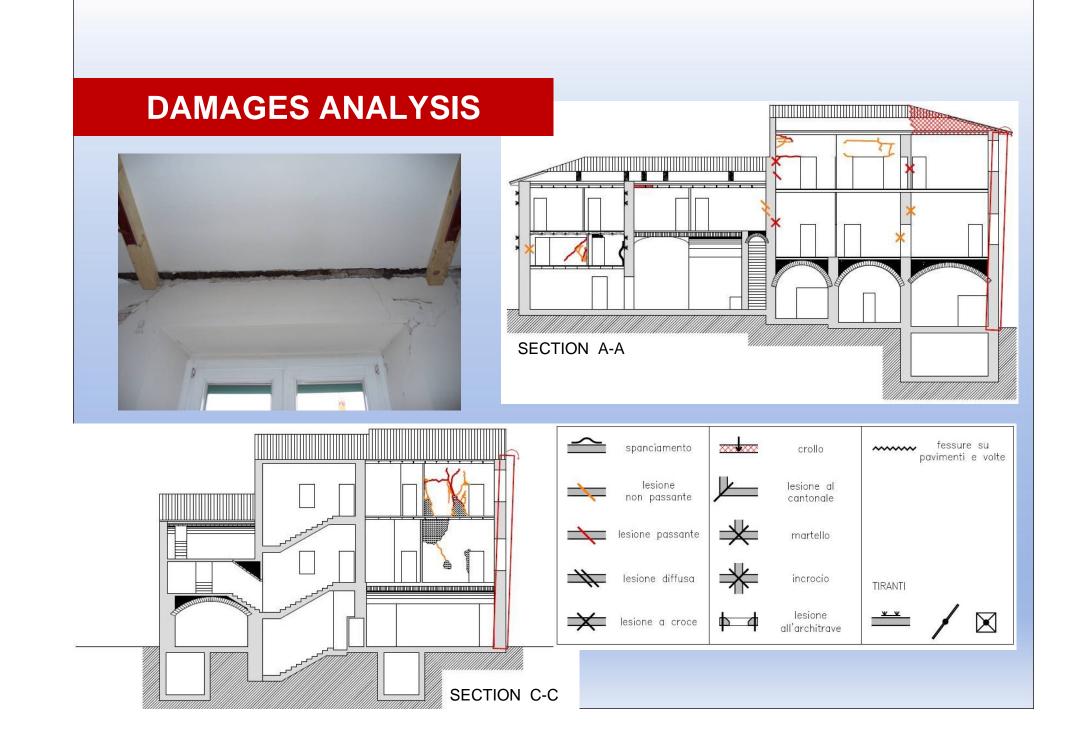


MIXED BEAM-VAULT FLOOR SLAB



BARREL VAULT





DAMAGES ANALYSIS





REINFORCED PLASTER



MASONRY WALLS

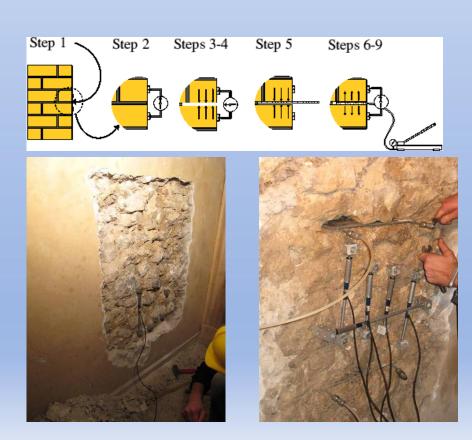


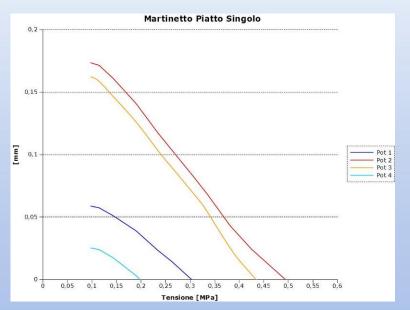
TIE RC BEAMS INSERTION

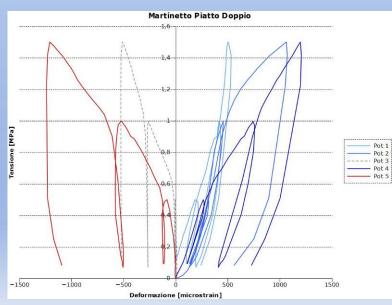
IN SITU TESTS

FLAT JACKS

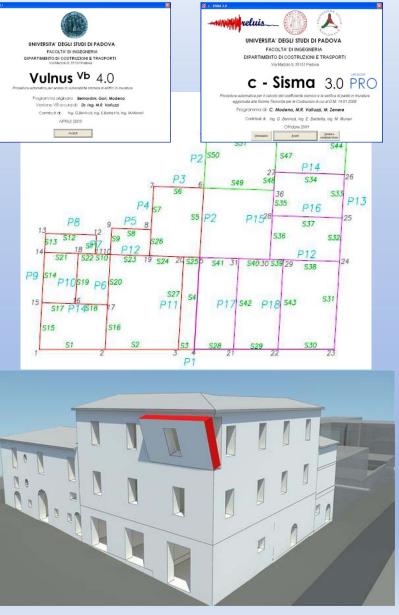
- <u>SINGLE</u>: stress in the masonry
- <u>DOUBLE</u>: elastic properties







STRUCTURAL BEHAVIOR ANALYSIS INVERSITA' DEGLI STUDI DI PADOVA FACOLTA' DI INGEGNERIA TIMENTO DI COSTRUZIONI E TRASPORT Vulnus Vb 4.0 Programma originario Bernardini, Gari, Mode ANALYSIS OF LOCAL MECHANISM Venione VB a cura d: Dr. Ina. M.R. Valluzzi Contribuil de lina, G.Beolincà, Ina, E.Barbetta, Ina, M APRILE 2009 Assessment of masonry building vulnerability. Avanti Calcolo indici |1 e |2 - v_1 Caratteristiche e risultati parziali di edifici Campione : P9 514 P10519 D6 520 Edificio Numero edificio. S17 P14S18 Materiale pareti Pietrame Indice 1 (Vet) direzione X: 0,233 Conservazione : Mediocre Indice 1 (Vet) direzione Y : 0.33 MPa Resistenza a compressione 2,6 Indice 2 min : 0,224 Setto: 30 Parete : 1 Resistenza a trazione MPa 0.14 kg/mc Indice 2" + indice 2" min : 0,106 Densità specifica equivalente (PSI = 0): 2370 + 0,118 Numero piani 3 Indice 2 max : 1,281 Parete : 15 Setto : 36 Solai Leggero Indice 2' + indice 2" max : 0,212 + 1,07 Pianta : Irregolare Indice 2 valore medio : 0,725 Altezza alla gronda: 12.44 Numero incatenamenti su prospetto //X: 0 Numero incatenamenti su prospetto // Y : 0 Calcola indici per ciascun setto Numero cordoli perimetrali 0 kN/m Contenimento pareti //X: 3,241 Salva / Stampa Risultati Contenimento pareti // Y : 9,724 kN/m Avanti -> <- Indietro Chiudi



VERTICAL MEMBERS

OUT OF PLANE MECHANISMS: Facades' Overturnng Bolzoni in acciaio 40x40 mm L=100 cm Profilo in acciaio d. 30 αP2 h₂ Profilo in accialo d. 30 d. 30 m 40x40 mm L=10 Prospetto rospetto Sezione Figura B.12.1 - Schematizzazione dell'ancoraggio con paletto o bolzone d SEZIONE ΡΙΑΝΤΑ PROSPETTO αP_1 h P PERFORAZIONI ARMATE CON B. DI ACCIAIO INDX #20 **INSERTION OF TIES**

VERTICAL MEMBERS

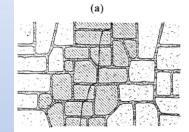
SHEAR CRACKS





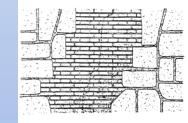
LOW QUALITY OF RESISTING WALL





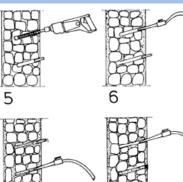
-1	0)))))	5	
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(b)



1	





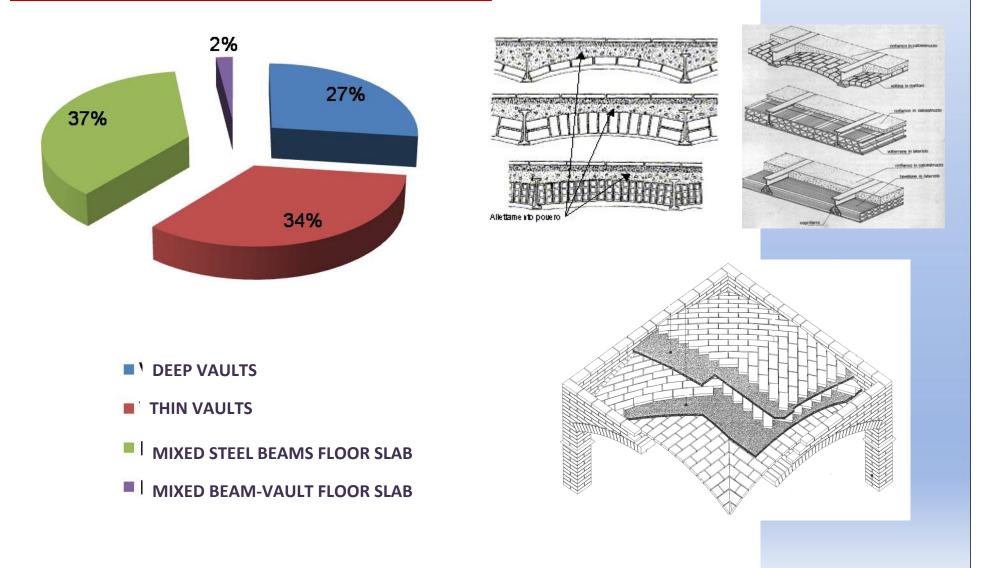


INJECTIONS

8

9

FLOOR SLABS

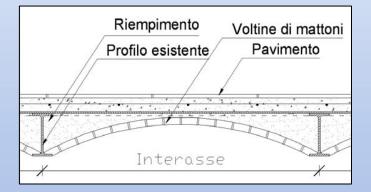


CRACKS AND PARTIAL OR TOTAL FLOOR SLABS COLLAPSE

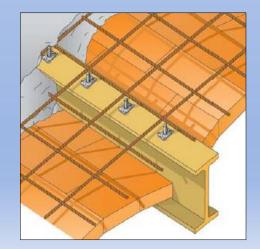




FLOOR SLABS



Floor slab strengthening with reinforced concrete slab







FLOOR SLABS

CRACKS OR PARTIAL COLLAPSE OF VAULTS



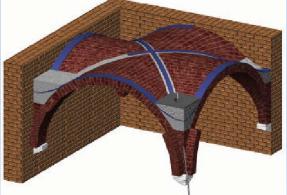




Strengthening with composites (FRP)





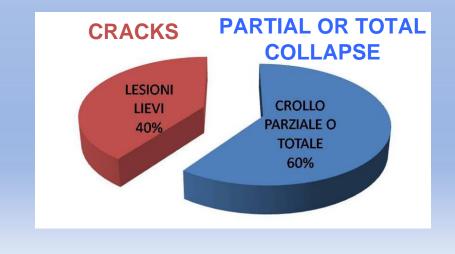


FLOOR SLABS

TOTAL COLLAPSE OF VAULTS



THIN VAULTS





DEEP VAULTS

RESTARTING FROM L'AQUILA



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